

**LAND TITLE ACT
FORM C (Section 233) CHARGE**

GENERAL INSTRUMENT - PART 1 Province of British Columbia

PAGE OF PAGES

Your electronic signature is a representation that you are a subscriber as defined by the Land Title Act, RSBC 1996 c.250, and that you have applied your electronic signature in accordance with Section 168.3, and a true copy, or a copy of that true copy, is in your possession.

1. APPLICATION: (Name, address, phone number of applicant, applicant's solicitor or agent)

Deduct LTSA Fees? Yes

2. PARCEL IDENTIFIER AND LEGAL DESCRIPTION OF LAND:
[PID] [LEGAL DESCRIPTION]

STC? YES

3. NATURE OF INTEREST

CHARGE NO.

ADDITIONAL INFORMATION

4. TERMS: Part 2 of this instrument consists of (select one only)

(a) Filed Standard Charge Terms D.F. No.

(b) Express Charge Terms Annexed as Part 2

A selection of (a) includes any additional or modified terms referred to in Item 7 or in a schedule annexed to this instrument.

5. TRANSFEROR(S):

6. TRANSFEREE(S): (including postal address(es) and postal code(s))

7. ADDITIONAL OR MODIFIED TERMS:

8. EXECUTION(S): This instrument creates, assigns, modifies, enlarges, discharges or governs the priority of the interest(s) described in Item 3 and the Transferor(s) and every other signatory agree to be bound by this instrument, and acknowledge(s) receipt of a true copy of the filed standard charge terms, if any.

Officer Signature(s)

Execution Date

Transferor(s) Signature(s)

Y	M	D

OFFICER CERTIFICATION:

Your signature constitutes a representation that you are a solicitor, notary public or other person authorized by the *Evidence Act*, R.S.B.C. 1996, c.124, to take affidavits for use in British Columbia and certifies the matters set out in Part 5 of the *Land Title Act* as they pertain to the execution of this instrument.

**LAND TITLE ACT
FORM D**

EXECUTIONS CONTINUED

PAGE of PAGES

Officer Signature(s)

Execution Date

Transferor / Borrower / Party Signature(s)

Y	M	D

OFFICER CERTIFICATION:

Your signature constitutes a representation that you are a solicitor, notary public or other person authorized by the *Evidence Act*, R.S.B.C. 1996, c.124, to take affidavits for use in British Columbia and certifies the matters set out in Part 5 of the *Land Title Act* as they pertain to the execution of this instrument.

SECTION 219 COVENANT—DEVELOPMENT COVENANT

TERMS OF INSTRUMENT—PART 2

THIS AGREEMENT dated for reference _____ is

BETWEEN:

TCD DEVELOPMENTS (GIBSONS) LTD., BC1089556, a British Columbia corporation having a registered office at 2827 West 43rd Avenue, Vancouver, B.C., V6N 3H9

(the “Owner”)

AND:

THE TOWN OF GIBSONS, located at 474 South Fletcher Road, Box 340, Gibsons, B.C., V0N 1V0

(the “Town”)

GIVEN THAT:

- A. The Owner is the registered owner in fee simple of the land in Gibsons, British Columbia, legally described as: BLOCK 9 EXCEPT: FIRSTLY; PART IN REFERENCE PLAN 18037, SECONDLY; PART SUBDIVIDED BY PLAN LMP21605, DISTRICT LOT 1328 GROUP 1 NEW WESTMINSTER DISTRICT PLAN 4014 (the “Land”);
- B. It is proposed that the Land be developed for the purpose of selling individual townhomes and condominiums;
- C. The Owner has asked the Town to accept the covenant created by this Agreement as a condition of rezoning; and
- D. The Owner wishes to grant, and the Town accepts, the s. 219 covenant contained in this Agreement over the Land.

THIS AGREEMENT is evidence that in consideration of payment of \$1.00 by the Town to the Owner (the receipt of which is acknowledged by the Owner), and in consideration of the promises exchanged below, the Owner covenants and agrees with the Town in accordance with s. 219 of the *Land Title Act* as follows:

1. The Owner covenants and agrees with the Town that:
 - (a) the development will substantially comply with the Site Plan, attached as Schedule A.
 - (b) prior to the issuance of a Form and Character Development Permit, the Owner will:
 - i. provide a \$150,000 payment to the Town as a Community Amenity Contribution; and

- ii. provide a \$270,000 payment to the Town as an Affordable Housing Contribution.
- (c) prior to issuance of a Building Permit, the Owner will:
 - i. register a statutory Right-of-Way, creating pedestrian and cycle access that connects Eaglecrest Drive and Stewart Road;
 - ii. Provide a security deposit to implement the recommendations in the Traffic Impact Study completed by Creative Transportation Solutions Ltd. and dated December 2017, attached as Schedule B, and as approved by the Director of Infrastructure Services, in the same manner as off-site improvements required by the *Town's Subdivision and Development Servicing and Stormwater Management Bylaw 1175, 2012* including:
 - 1. constructing a curb bulb-out in the northwest quadrant and the northeast quadrant of the intersection of Eaglecrest Drive and Inglis Road;
 - 2. constructing a crosswalk with sidewalk letdowns on the north side of Eaglecrest Drive; and
 - 3. connecting the sidewalk from the site at the lower bench to the existing sidewalk on Winn Road at Abbs Road.
 - iii. survey and register a blanket statutory Right-of-Way over the greenspace for the purpose of public access shown on Plan attached as Schedule C.
- (d) prior to issuance of occupancy for final dwelling, the Owner will:
 - i. register Right-of-Ways after construction and survey of trails; and
 - ii. install evergreen screening along the North property line;
- (e) no building will be constructed to a height exceeding 8 metres except for two buildings located to the south of the third row, shown on the Site Plan diagram attached as Schedule C.

2. The Town will execute and deliver to the Owner a discharge, in registrable form, of this Agreement from title to the Land, at the expense of the Owner, if the Owner has at its expense completed all things necessary to comply with section 1 to the satisfaction of the Town.

3. Any opinion, decision, act or expression of satisfaction provided for in this Agreement is to be taken or made by the Town's Director of Planning or his or her delegate authorized as such in writing.

4. The Owner releases, and must indemnify and save harmless, the Town, its elected and appointed officials and employees, from and against all liability, actions, causes of action, claims, damages, expenses, costs, debts, demands or losses suffered or incurred by the Owner, or anyone else, arising from the granting or existence of this Agreement, from the performance by the Owner of this Agreement, or any default of the Owner under or in respect of this Agreement.

5. The parties agree that this Agreement creates only contractual obligations and obligations arising out of the nature of this document as a covenant under seal. The parties agree that no tort obligations or liabilities of any kind exist between the parties in connection with the performance of, or any default under or in respect of, this Agreement. The intent of this section is to exclude tort liability of any kind and to limit the parties to their rights and remedies under the law of contract and under the law pertaining to covenants under seal.

6. The rights given to the Town by this Agreement are permissive only and nothing in this Agreement imposes any legal duty of any kind on the Town to anyone, or obliges the Town to enforce this Agreement, to perform any act or to incur any expense in respect of this Agreement.

7. This Agreement does not

- (a) affect or limit the discretion, rights or powers of the Town under any enactment (as defined in the *Interpretation Act*, R.S.B.C. 1996, c. 238, on the reference date of this Agreement) or at common law, including in relation to the use or subdivision of the Land,
- (b) affect or limit any enactment relating to the use or subdivision of the Land, or
- (c) relieve the Owner from complying with any enactment, including in relation to the use or subdivision of the Land.

8. Every obligation and covenant of the Owner in this Agreement constitutes both a contractual obligation and a covenant granted under s. 219 of the *Land Title Act* in respect of the Land and this Agreement burdens the Land and runs with it and binds the successors in title to the Land. This Agreement burdens and charges all of the Land and any parcel into which it is subdivided by any means and any parcel into which the Land is consolidated. The Owner is only liable for breaches of this Agreement that occur while the Owner is the registered owner of the Land.

9. The Owner agrees to do everything reasonably necessary, at the Owner's expense, to ensure that this Agreement is registered against title to the Land with priority over all financial charges, liens and encumbrances registered, or the registration of which is pending, at the time of application for registration of this Agreement.

10. An alleged waiver of any breach of this Agreement is effective only if it is an express waiver in writing of the breach in respect of which the waiver is asserted. A waiver of a breach of this Agreement does not operate as a waiver of any other breach of this Agreement.

11. If any part of this Agreement is held to be invalid, illegal or unenforceable by a court having the jurisdiction to do so, that part is to be considered to have been severed from the rest of this Agreement and the rest of this Agreement remains in force unaffected by that holding or by the severance of that part.

12. This Agreement is the entire agreement between the parties regarding its subject.

13. This Agreement binds the parties to it and their respective successors, heirs, executors and administrators.

14. The Owner must do everything reasonably necessary to give effect to the intent of this Agreement, including execution of further instruments.

15. By executing and delivering this Agreement each of the parties intends to create both a contract and a deed executed and delivered under seal.

As evidence of their agreement to be bound by the above terms, the parties each have executed and delivered this Agreement under seal by executing Part 1 of the *Land Title Act* Form C to which this Agreement is attached and which forms part of this Agreement.

Schedule A—Site Plan

Schedule B- Traffic Impact Study

Schedule C—Site Plan Diagram showing the Blanket SRW and the Two Buildings over 8 metres in height

END OF DOCUMENT

SCHEDULE B

464 Eaglecrest Drive Development Traffic Impact Study

Prepared for:

TCD Development Group



Prepared by:



DECEMBER 2017

84a moody street
port moody, bc
canada v3h 2p5

604.936.6190

604.936.6175

www.cts-bc.com

Date: 12 December 2017

Our File No: 5602-01

BY EMAIL

Mr. Rob Chetner
TCD Developments
1754 West 3rd Avenue
Vancouver, BC,
V6J 1K4

Dear Mr. Chetner

Re: 464 Eaglecrest Drive Development Traffic Impact Study, Sechelt, BC

Creative Transportation Solutions Ltd. (CTS) is pleased to submit this report summarising our work on the above study. CTS was commissioned to undertake a traffic impact assessment of the proposed residential development located at 464 Eaglecrest Drive in the Town of Gibsons. This letter report documents the analysis and findings of the study.

1.0 BACKGROUND

The proposed residential development will consist of 87 multi-family units (63 unit in the upper site and 24 units in the lower site) and is located on the east side of Eaglecrest Drive as illustrated on **FIGURE 1**. A copy of the site plan is included in **APPENDIX A**. The steep topography of this site makes the provision for an internal road connecting the entire site extremely difficult and if constructed, would result in very large retaining walls and removal of significant natural terrain. Therefore, the provision of one driveway connecting to Eaglecrest Drive opposite Inglis Road to access the upper portion of the site and a second driveway connecting to Stewart Road to access the lower portion of the site is being proposed.

The design hour of analysis for traffic impact studies on the Sunshine Coast is the Friday afternoon peak hour, which is when traffic volumes are typically their highest. This is due to it being a combination of commuters heading home, local shopping trips and the influx of weekend visitors arriving for the weekend from the late afternoon BC Ferries departure from Horseshoe Bay.

**FIGURE 1
STUDY AREA & ROAD NETWORK**



2.0 EXISTING CONDITIONS

2.1 Study Area

The study area is bounded by O'Shea Road to the north, Gower Point Road to the east, Winn Road to the south, and Shaw Road to the west. The following intersections were included in the traffic impact assessment:

- 1) Shaw Road & Inglis Road (*unsignalized*);
- 2) Eaglecrest Drive & Inglis Road (*unsignalized*);
- 3) Eaglecrest Drive & O'Shea Road (*unsignalized*);
- 4) Winn Road & Abbs Road (*unsignalized*);
- 5) Winn Road & Gower Point Road (*unsignalized*); and
- 6) All site Accesses.

2.2 Traffic Volumes

CTS conducted intersection traffic movement counts on Friday, 13 October 2017 from 14:00 to 17:30 in order to capture both school peak and residential peak periods when the adjacent road network is most congested. Also, schools were in session and there were no significant ferry disruptions that day which could have negatively impacted traffic volumes in the study area. The traffic count data was tabulated and reviewed to ensure data integrity and validity. The tabulated traffic movement count data sheets are in **APPENDIX B**.

For the purpose of this study, the collected October data was factored up to represent estimated peak August volumes in Gibsons. **FIGURE 2** illustrates the daily traffic volume on Highway 101 for day of week from the permanent count station maintained by the Ministry of Transportation & Infrastructure, which shows that Fridays is the busiest day of the week. **FIGURE 3** illustrates the traffic volumes between October and August data and the calculated seasonal adjustment factor of 12%. The October data was then factored up by 1.12 (12%) to estimate the design traffic volumes for the study area. **FIGURE 4** illustrates the estimated August Friday afternoon peak hour vehicle volumes respectively for the year 2017.

FIGURE 2
AUGUST 2016 DAILY TRAFFIC VOLUMES BY DATE OF WEEK
(MOTI's PERMANENT COUNT STATION)

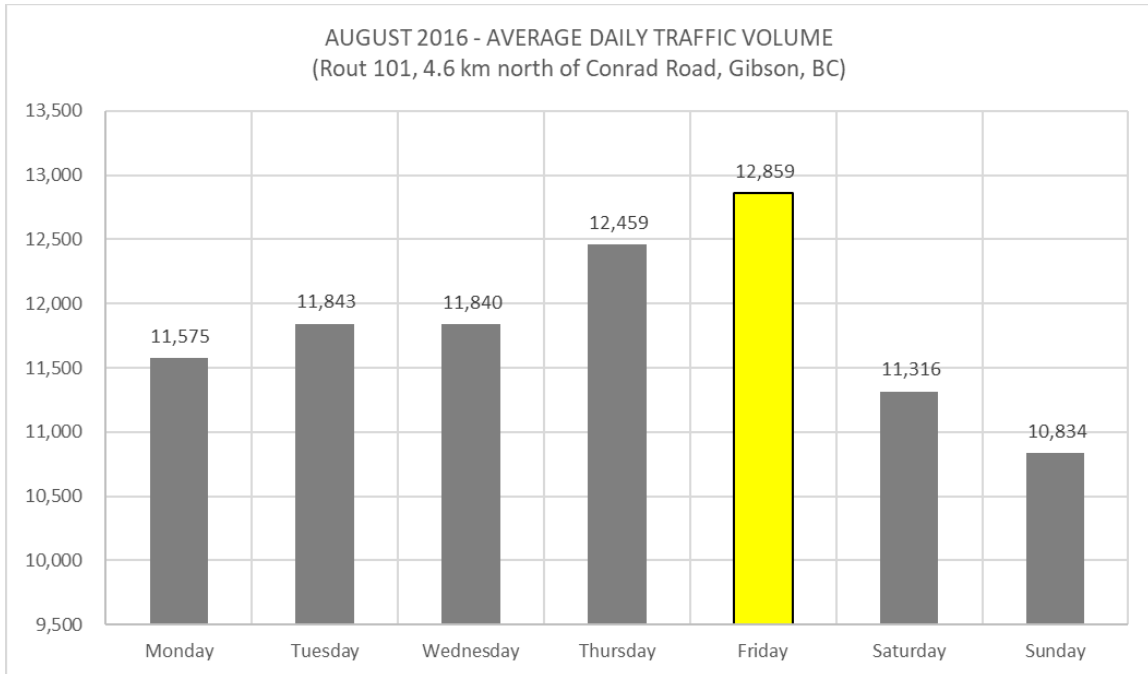


FIGURE 3
2016 DAILY FRIDAY TRAFFIC VOLUMES BY MONTH
(MOTI's PERMANENT COUNT STATION)

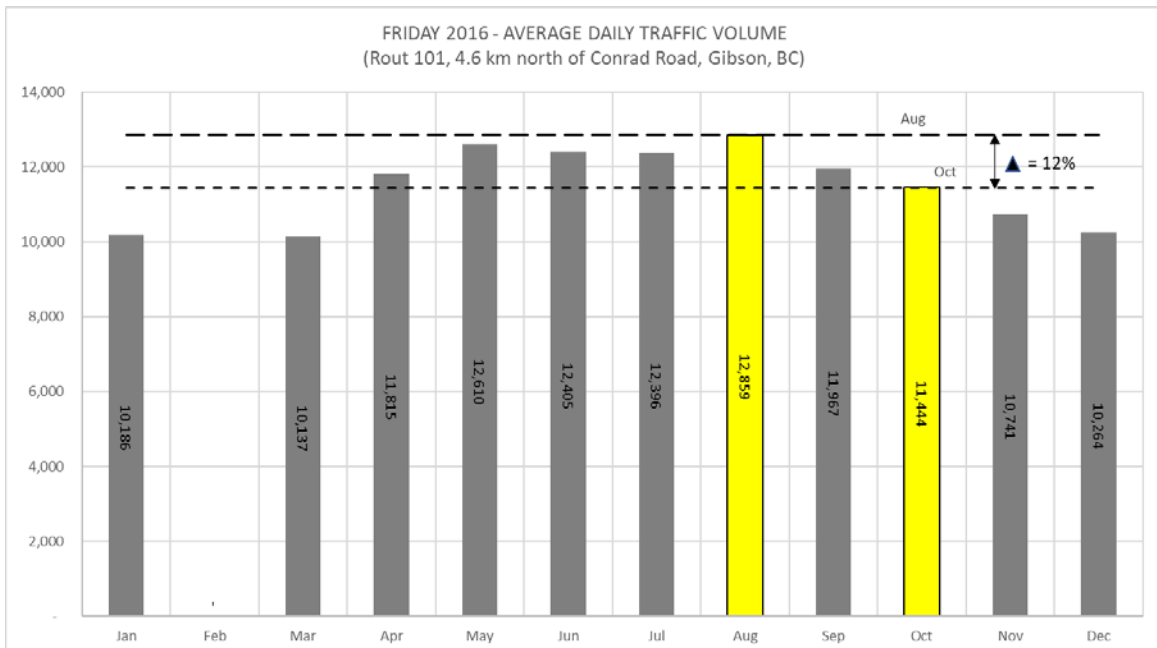
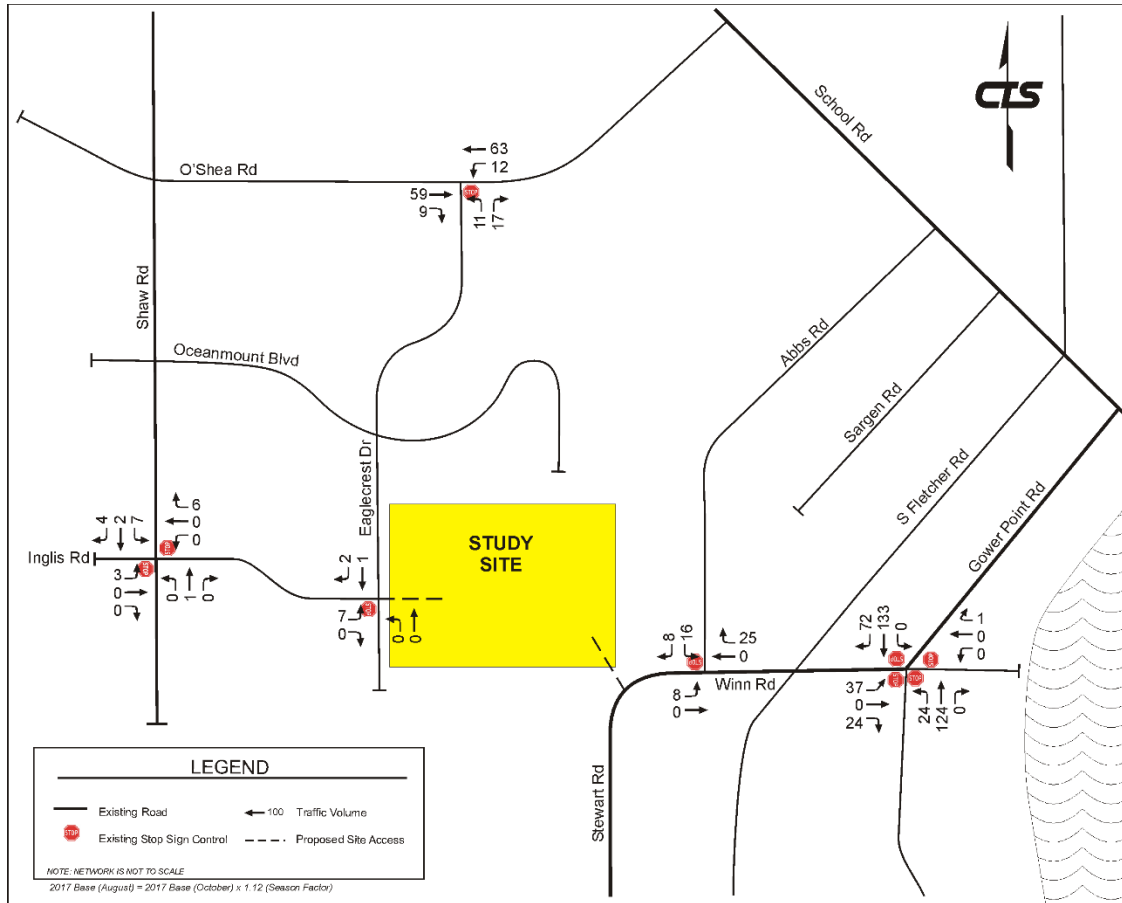


FIGURE 4
2017 ESTIMATED AUGUST FRIDAY AFTERNOON
PEAK HOUR BASE TRAFFIC VOLUMES

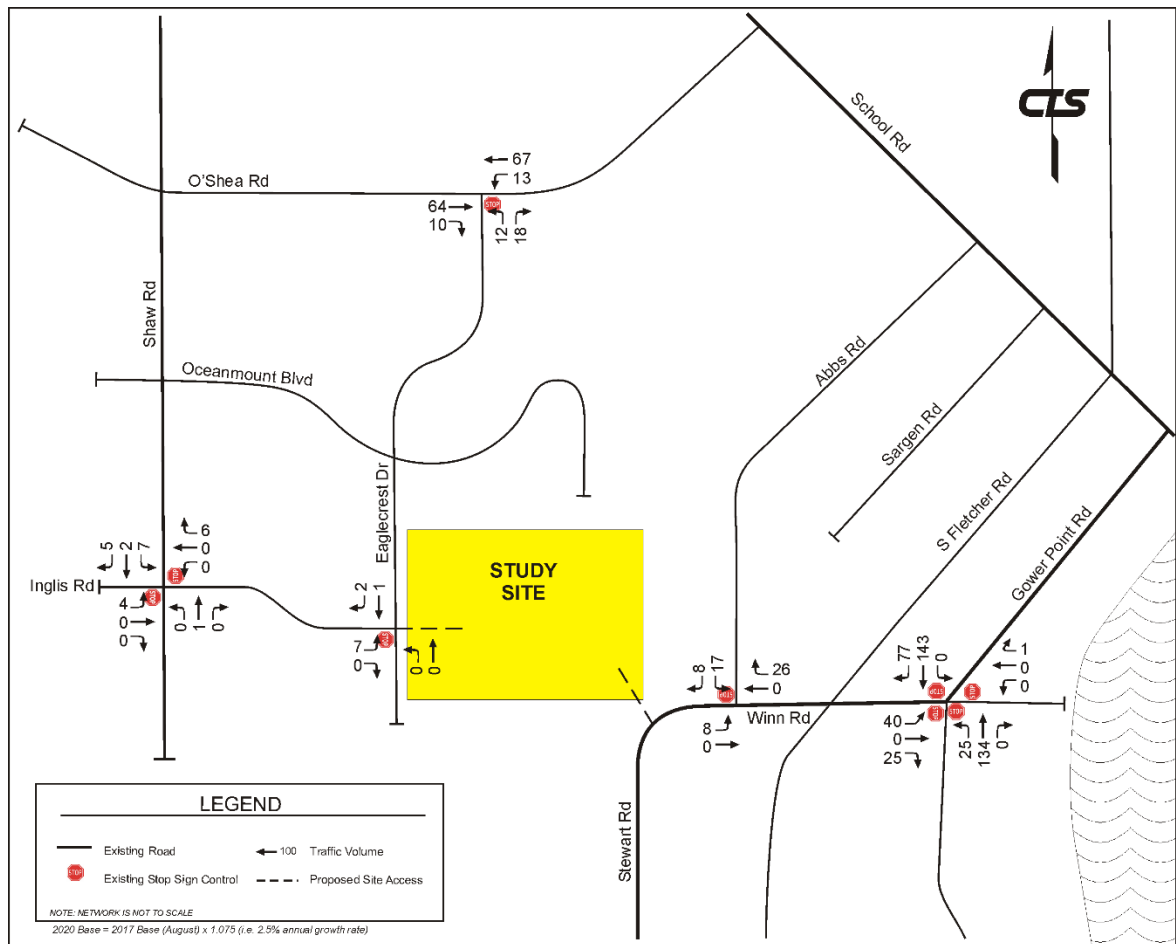


3.0 FUTURE BASE TRAFFIC VOLUMES

2020 Future Base Traffic Volumes

Year 2020 is anticipated to be the year of full buildout for the proposed development. Therefore, in order to access the traffic impacts of the proposed development on the base traffic volumes, the 2017 base traffic volumes were factored up by a traffic volume growth rate of 2.5% per annum (simple straight line) to represent the year 2020 base traffic volumes. **FIGURE 5** illustrates the projected 2020 Friday afternoon peak hour volumes for the future base conditions with no development traffic.

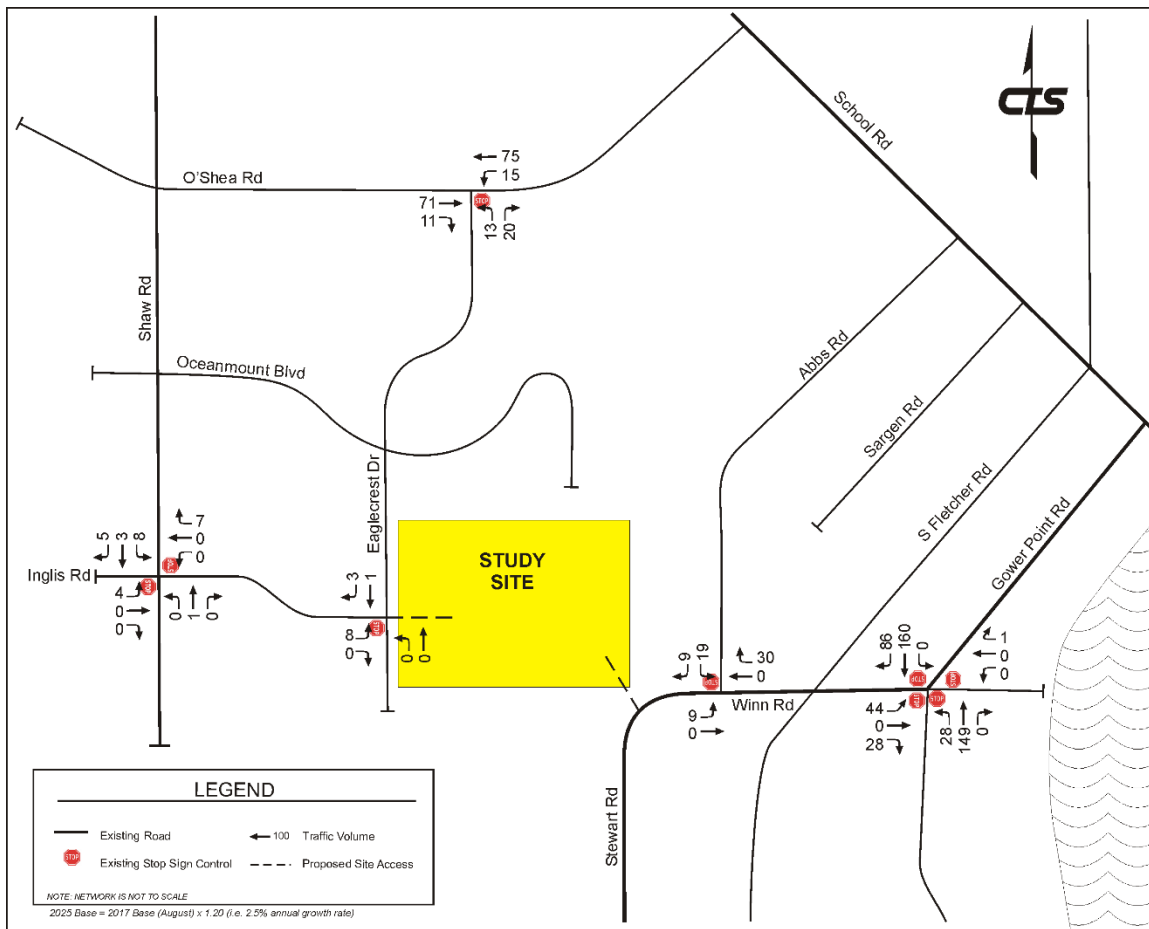
**FIGURE 5
2020 ESTIMATED AUGUST FRIDAY AFTERNOON
PEAK HOUR BASE TRAFFIC VOLUMES**



2025 Future Base Traffic Volumes

CTS also examined the year 2025 as this represents 5 years “post-buildout”. The summer 2017 base traffic volumes were factored up by a traffic volume growth rate of 2.5% per annum (simple straight line) to represent the year 2025 base traffic volumes. **FIGURE 6** illustrates the projected 2025 Friday afternoon peak hour volumes for the future base conditions with no development traffic.

**FIGURE 6
2025 ESTIMATED AUGUST FRIDAY AFTERNOON
PEAK HOUR BASE TRAFFIC VOLUMES**



4.0 SITE TRAFFIC VOLUME

4.1 Traffic Generation

The published vehicle trip generation rates from the Institute of Transportation Engineers (ITE) *Traffic Generation Manual 10th Edition* were used to forecast the site generated traffic volumes in accordance with MOTI requirements and current accepted traffic engineering standards. **TABLE 1** summarizes the forecast site generated traffic for the proposed development. Of note, internal trip making credits and pass-by traffic were all assumed to be zero so that the projected volumes would represent the worst case scenario such that all the traffic would be “new” traffic to the adjacent road network.

**TABLE 1
SUMMARY OF SITE GENERATED TRAFFIC**

Land Use	Peak Hour	Trip Generation Variable	Horizon Year	Scope of Development	Vehicle Trip Generation Rate	Trip Rate Source	Directional Split		Peak Hour Volumes (vph)		
							% in	% out	in	out	total
Upper Block - Townhouse	Weekday Morning	Dwelling Units	2020	16	0.46	ITE 10th Edition, Code (220)	23%	77%	2	6	8
	Weekday Afternoon				0.56		63%	37%	6	3	9
Upper Block - Condos	Weekday Morning	Dwelling Units	2020	47	0.46	ITE 10th Edition, Code (220)	23%	77%	5	17	22
	Weekday Afternoon				0.56		63%	37%	17	10	27
Lower Block - Condos	Weekday Morning	Dwelling Units	2020	24	0.46	ITE 10th Edition, Code (220)	23%	77%	3	9	12
	Weekday Afternoon				0.56		63%	37%	9	5	14
TOTAL WEEKDAY MORNING PEAK HOUR									10	32	42
TOTAL WEEKDAY AFTERNOON PEAK HOUR									32	18	50

The proposed development is forecast to 42 vehicle trips during the weekday morning peak hour (i.e. 10 inbound and 32 outbound) and 50 vehicle trips during the weekday afternoon peak hour (i.e. 32 inbound and 18 outbound). This is equivalent to an average of 1 vehicle movement every 1.2 minutes combined for both driveways, which from a traffic engineering point of view is not considered significant.

Of note, the threshold of the Government of British Columbia for determining when a development triggers a formal traffic impact assessment is when a site generates 100 or more vehicles during the design hour. Therefore, the proposed development does not meet this requirement as the maximum volume forecast is only 50 vehicles, which is 50% of the warrant. This confirms that the site generated traffic volumes for the site are negligible and any traffic impacts associated with this development would be localized to just the site access

4.2 Trip Distribution

Trip distribution parameters to distribute the site generated vehicle trips to/from the site were developed from existing traffic patterns entering and exiting the study area for the weekday afternoon peak hour. The trip distribution parameters used in this study are summarized in **TABLE 2A** and **TABLE 2B** while the associated traffic volumes assignment is summarized in **TABLE 3A** and **TABLE 3B**.

**TABLE 2A
TRIP DISTRIBUTION PERCENTAGES
FOR UPPER SITE GENERATED TRAFFIC**

FROM/ TO	WEEKDAY AFTERNOON PEAK HOUR	
	INBOUND	OUTBOUND
North - Shaw Rd	8.6%	6.3%
East - O'Shea Rd	47.9%	47.6%
West - O'Shea Rd	43.6%	46.2%

**TABLE 2B
TRIP DISTRIBUTION PERCENTAGES
FOR LOWER SITE GENERATED TRAFFIC**

FROM/ TO	WEEKDAY AFTERNOON PEAK HOUR	
	INBOUND	OUTBOUND
North - Abbs Rd	6.1%	9.0%
North - Gower Point Rd	53.4%	45.2%
South - Gower Point Rd	38.5%	43.6%
South - Stewart Rd	2.0%	2.2%

**TABLE 3A
TRIP DISTRIBUTION VEHICLE VOLUMES
FOR UPPER SITE GENERATED TRAFFIC**

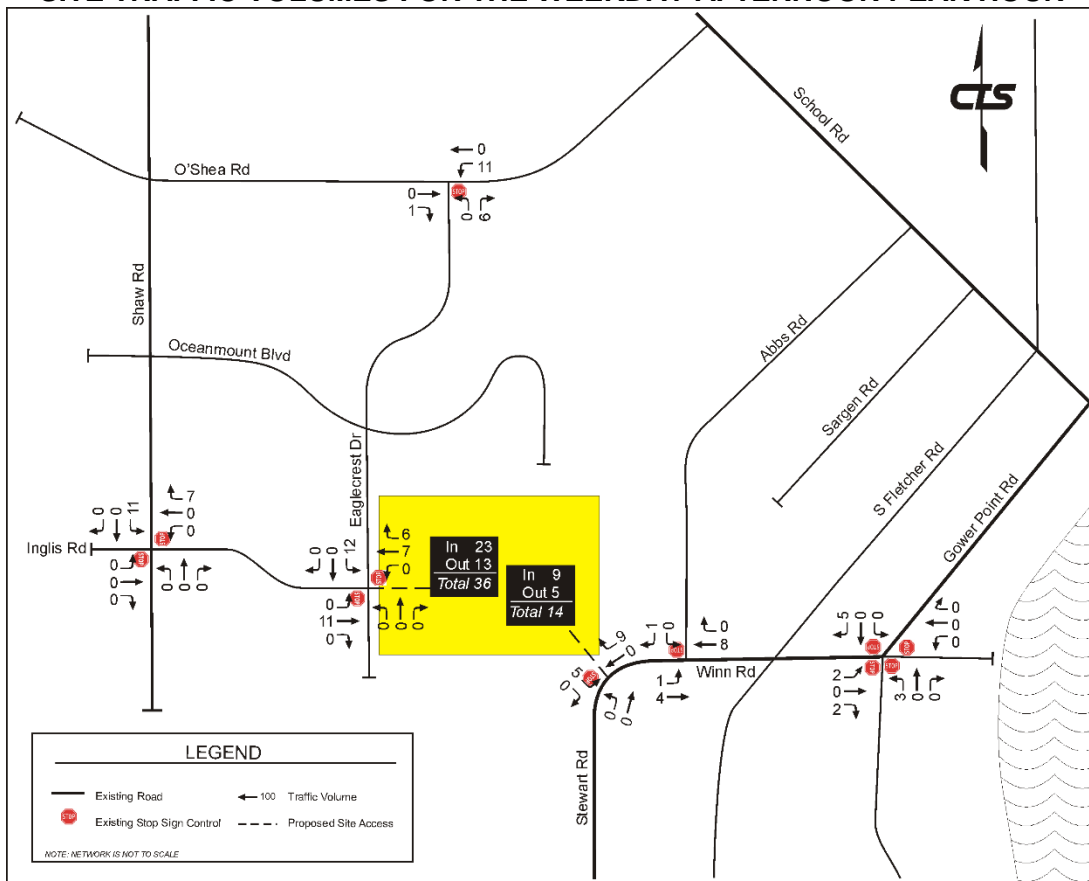
FROM / TO	WEEKDAY AFTERNOON PEAK HOUR	
	INBOUND	OUTBOUND
North - Shaw Rd	2	1
East - O'Shea Rd	11	6
West - O'Shea Rd	10	6
TOTAL	23	13
	36	

**TABLE 3B
TRIP DISTRIBUTION VEHICLE VOLUMES
FOR LOWER SITE GENERATED TRAFFIC**

FROM / TO	WEEKDAY AFTERNOON PEAK HOUR	
	INBOUND	OUTBOUND
North - Abbs Rd	1	1
North - Gower Point Rd	5	2
South - Gower Point Rd	3	2
South - Stewart Rd	0	0
TOTAL	9	5
	14	

FIGURE 7 illustrates the projected site generated volumes on the road network for buildout in the year of 2020 weekday afternoon peak hour respectively.

**FIGURE 7
SITE TRAFFIC VOLUMES FOR THE WEEKDAY AFTERNOON PEAK HOUR**



5.0 TOTAL PROJECTED TRAFFIC VOLUMES

FIGURE 8 illustrates the total projected traffic for the year 2020 Friday afternoon peak hour consisting of both future base and site traffic resulting from the proposed development. It is the result of superimposing **FIGURE 7** onto **FIGURE 5**.

FIGURE 9 illustrates the total projected traffic for the year 2025 Friday afternoon peak hour consisting of both future base and site traffic resulting from the proposed development. It is the result of superimposing **FIGURE 7** onto **FIGURE 6**.

FIGURE 8
2020 FRIDAY AFTERNOON PEAK HOUR BASE + SITE TRAFFIC VOLUMES

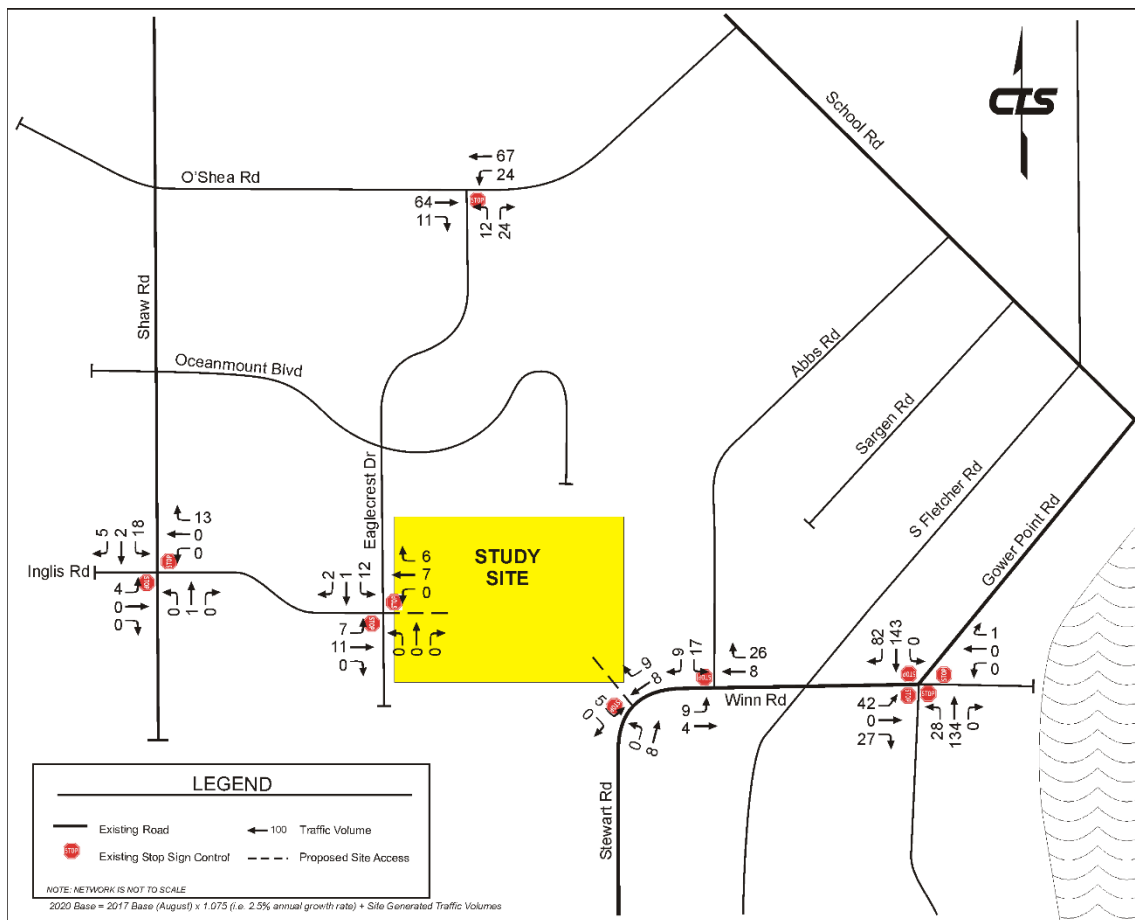
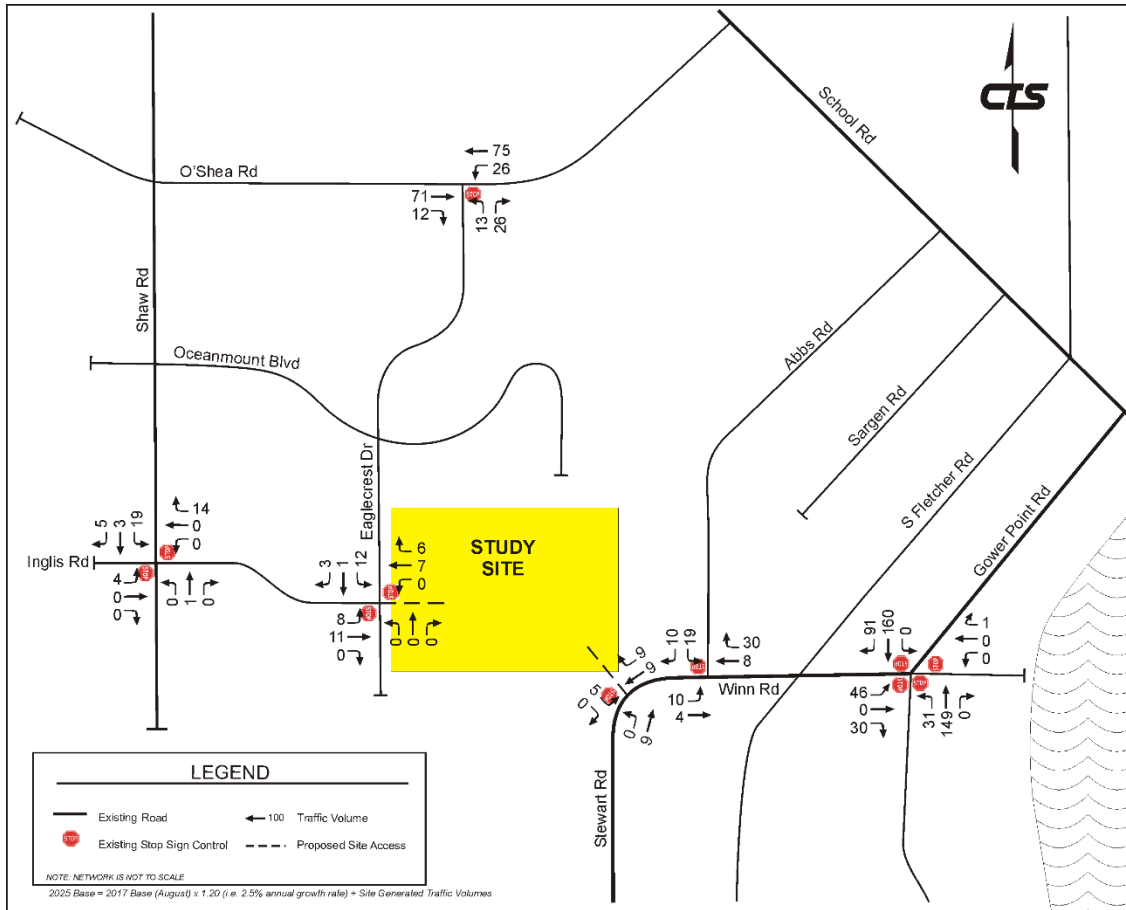


FIGURE 9
2025 FRIDAY AFTERNOON PEAK HOUR BASE + SITE TRAFFIC VOLUMES



6.0 TRAFFIC ENGINEERING ANALYSIS

6.1 Intersection Capacity Analysis

Capacity analysis was performed at each of the locations in order to determine the intersection levels of service (LOS) that is provided to motorists. LOS for intersections is defined in terms of delay (seconds per vehicle), which is a measure of driver discomfort and frustration, fuel consumption and lost travel time.

An intersection or movement LOS can range from "A" (which is excellent) to "E" (which is capacity). A LOS of "F" indicates that an intersection or movement capacity is failing because vehicle delays are excessive. A LOS of "D" during the critical peak hours for unsignalized intersections is considered acceptable by the Ministry of Transportation & Infrastructure for overall intersection operation.

Volume to capacity (v/c) ratios typically ranges from 0.25 to 1.20 with a v/c ratio of 1.0 indicating the movement, approach or intersection is at capacity.

Highway Capacity Software 2017 (HCS 7) was used for the unsignalized intersection analysis. The following assumptions were made with respect to the intersection capacity analysis:

- Saturation flow rate = 1,800 passenger cars/hour of green time/lane (pcphgpl)
- Heavy vehicle percentage for all roads = 2%
- Peak hour factor (PHF) = 0.82 in the afternoon peak hour, which are based on the average PHF from the intersections surveyed.

TABLE 4 summarizes and compares the main performance parameters of the intersection capacity analysis for unsignalized intersections. Delay time in seconds for each lane group is summarized for unsignalized intersections. Wherever necessary, attempts at improvements have been made to maintain intersection and approach movement level of service standards for each of the post-development scenarios. The capacity analysis worksheets are included in **APPENDIX C**.

**TABLE 4
VEHICLE DELAY BY INDIVIDUAL MOVEMENTS FOR UNSIGNALIZED INTERSECTIONS**

INTERSECTION	TIME OF DAY	SCENARIO	PERFORMANCE MEASURE	EASTBOUND			WESTBOUND			NORTHBOUND			SOUTHBOUND			LOS	NOTES	
				Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right			
Shaw Road (N/S) & Inglis Road (E/W)	Weekday Afternoon Peak Hour	2017 Base	Volumes	3	0	0	0	0	6	0	1	0	7	2	4	A	Okay.	
			Delay	8.8			8.4			7.2			7.2					
		2020 Base	Volumes	4	0	0	0	0	6	0	1	0	7	2	5	A	Okay.	
			Delay	8.8			8.4			7.2			7.2					
		2020 Base + Site	Volumes	4	0	0	0	0	13	0	1	0	18	2	5	A	Okay.	
			Delay	9.0			8.4			7.2			7.3					
		2025 Base + Site	Volumes	4	0	0	0	0	14	0	1	0	19	3	5	A	Okay.	
			Delay	9.1			8.4			7.2			7.3					
Eaglecrest Drive (N/S) & Inglis Road (E/W)	Weekday Afternoon Peak Hour	2017 Base	Volumes	7		0				0	0			1	2	A	Okay.	
			Delay	8.6						7.2			0.0					
		2020 Base	Volumes	7		0				0	0				1	2	A	Okay.
			Delay	8.6						7.2			0.0					
		2020 Base + Site	Volumes	7	11	0	0	7	6	0	0	0	12	1	2	A	Okay.	
			Delay	9.1			8.9			7.2			7.2					
		2025 Base + Site	Volumes	8	11	0	0	7	6	0	0	0	12	1	3	A	Okay.	
			Delay	9.1			8.9			7.2			7.2					
Eaglecrest Drive (N/S) & O' Shea Road (E/W)	Weekday Afternoon Peak Hour	2017 Base	Volumes		59	9	12	63		11		17				A	Okay.	
			Delay	0.0			7.4			9.3								
		2020 Base	Volumes		64	10	13	67		12		18				A	Okay.	
			Delay	0.0			7.5			9.4								
		2020 Base + Site	Volumes		64	11	24	67		12		24				A	Okay.	
			Delay	0.0			7.5			9.5								
		2025 Base + Site	Volumes		71	12	26	75		13		26				A	Okay.	
			Delay	0.0			7.5			9.6								
Abbs Road (N/S) & Winn Road (E/W)	Weekday Afternoon Peak Hour	2017 Base	Volumes	8	0			0	25				16		8	A	Okay.	
			Delay	7.3						0.0			8.7					
		2020 Base	Volumes	8	0			0	26				17		8	A	Okay.	
			Delay	7.3						0.0			8.7					
		2020 Base + Site	Volumes	9	4			8	26				17		9	A	Okay.	
			Delay	7.3						0.0			8.8					
		2025 Base + Site	Volumes	10	4			8	30				19		10	A	Okay.	
			Delay	7.3						0.0			8.8					
Gower Point Road (N/S) & Winn Road (E/W)	Weekday Afternoon Peak Hour	2017 Base	Volumes	37	0	24	0	0	1	24	124	0	0	133	72	A	Okay.	
			Delay	8.3			7.4			8.7			8.7					
		2020 Base	Volumes	40	0	25	0	0	1	25	134	0	0	143	77	A	Okay.	
			Delay	8.4			7.5			8.8			9.0					
		2020 Base + Site	Volumes	42	0	27	0	0	1	28	134	0	0	143	82	A	Okay.	
			Delay	8.5			7.5			8.9			9.1					
		2025 Base + Site	Volumes	46	0	30	0	0	1	31	149	0	0	160	91	A	Okay.	
			Delay	8.7			7.6			9.3			9.5					
Site Access (N/S) & Winn Rd / Steward Rd	Weekday Afternoon Peak Hour	2020 Base + Site	Volumes	0	8			8	9				5		0	A	Okay.	
			Delay	7.3						0.0			8.8					
		2025 Base + Site	Volumes	0	9			9	9				5		0	A	Okay.	
			Delay	7.3						0.0			8.8					

Delay = Average Delay (seconds/vehicle)
 Intersection approaching capacity (LOS 'D' or 'E'); or medium approach delays (25sec to <50sec)
 Intersection equals or exceeds capacity (LOS 'F'); or high approach delays (>= 50sec)

All of the intersections and/or movements in this study area are currently operating or projected to operate at LOS A (excellent) for all scenarios tested and no operational and/or geometrical improvements are warranted.

However, it is recommended that on the north side of the intersection of Eaglecrest Drive & Inglis Road, a curb bulb-out be constructed in both the northwest quadrant and the northeast quadrant for the following reasons:

1. A narrowing of the cross section of Eaglecrest Drive will deter some motorists from using Eaglecrest Drive to access or egress the site as it reduces the vehicle capacity of that roadway; and
2. The narrowing of the cross section of Eaglecrest Drive also reduces the width of the crosswalk for pedestrians wishing to cross Eaglecrest Drive, which will reduce the exposure of pedestrians to vehicles.

With this geometric improvement, CTS is forecasting a 50/50 split on site traffic volumes using Eaglecrest Drive and Inglis Road (or 18 vehicles per hour during the design hour). This is equivalent to an additional vehicle movement on either Eaglecrest Drive or Inglis Road from the proposed development of 1 vehicle every 3 minutes. From a traffic engineering point of view, this is considered negligible.

7.0 CONCLUSIONS & RECOMMENDATIONS

7.1 Conclusions

- 1) CTS was retained by TCD Developments to prepare a traffic impact study for the proposed residential development at 464 Eaglecrest Drive in the Town of Gibsons, BC.
- 2) The current proposed development contains 87 multi-family units in the site (63 units in upper site and 24 units in lower site). One site access is provided to connect to Eaglecrest Drive opposite Inglis Road for the upper site while the second access connects to Stewart Road for the lower site. For the purposes of this study, construction was assumed to be completed and the residential units fully occupied by the year 2020.
- 3) CTS conducted turning movement counts on Friday, 13 October 2017. Schools were in session and there were no significant ferry disruptions that day which could have negatively impacted traffic volumes on Highway 101.
- 4) For the purposes of this study, the collected October data was factored up by a 1.12 factor to convert the Friday October data to peak August Friday data, which was derived by examining one year of traffic volume data on Highway 101 from the Ministry of Transportation & Infrastructure.
- 5) In order to be consistent with previous traffic studies done by CTS in Gibsons and which were approved by the Town of Gibsons, future base traffic volumes were

projected with a 2.5% annual traffic growth rate (simple straight line) to estimate future conditions. The design hour of analysis was the Friday afternoon peak hour.

- 6) The proposed development is forecast to generate a total of 50 vehicle trips during the weekday afternoon peak hour (32 inbound, 18 outbound), which is equivalent to an average of just less than 1 vehicle movement per minute. From a traffic engineering point of view, the projected traffic volume for the site is not considered significant.
- 7) The Ministry of Transportation & Infrastructure's warrant for requiring a traffic impact study for a proposed development is 100 or more new vehicle trips during any one hour period. Therefore, the proposed development does not meet this warrant as the projected driveway volumes are only 50% of the threshold and a formal traffic impact study is not technically warranted.
- 8) The capacity analysis determined that the existing intersections in the study area can accommodate the projected site traffic without any operational and/or geometrical improvements were warranted.
- 9) However, it is recommended that on the north side of the intersection of Eaglecrest Drive & Inglis Road, a curb bulb-out be constructed in both the northwest quadrant and the northeast quadrant for the following reasons:
 - A narrowing of the cross section of Eaglecrest Drive will deter some motorists from using Eaglecrest Drive to access or egress the site as it reduces the vehicle capacity of that roadway; and
 - The narrowing of the cross section of Eaglecrest Drive also reduces the width of the crosswalk for pedestrians wishing to cross Eaglecrest Drive, which will reduce the exposure of pedestrians to vehicles.

With this geometric improvement, CTS is forecasting a 50/50 split on site traffic volumes using Eaglecrest Drive and Inglis Road (18 vehicles per hour during the design hour). This is equivalent to an additional vehicle movement on either Eaglecrest Drive or Inglis Road from the proposed development of 1 vehicle every 3 minutes. From a traffic engineering point of view, this is considered negligible.

7.2 Recommendations

Based on the findings of this updated traffic impact study, the following is recommended:

1. That a crosswalk with sidewalk letdowns and curb bulb-outs be constructed on the north side of Eaglecrest Drive to link the site to the existing neighbourhood pedestrian network; and
2. That a sidewalk will need to be connected from the site at the lower bench to the existing sidewalk on Winn Road at Abbs Road to link the site to the pedestrian network of Lower Gibsons.

We would like to take this opportunity to thank you for this unique and challenging assignment and we look forward to working with you again in the future. Please call the undersigned should you have any questions or comments regarding this report.

Yours truly,

CREATIVE TRANSPORTATION SOLUTIONS LTD.



Jan O. Voss, M.Sc., P.Eng., PTOE
President

Attachment



APPENDIX A

Site Plan

1645 West 5th Avenue
Vancouver, BC V6J 1N5

Tel: (604) 872-2595 Fax: (604) 872-2505
Email: office@AMArchitects.com



Project:
1638

Eaglecrest Drive
461 Eaglecrest Drive, Gibsons, BC

Drawing:
Site Plan - New Cluster
Massing

Project Status:
RZ

SUBMISSION
DATE: _____
DESCRIPTION: _____

REVISION
No. Date Description

All Drawings in this set to be read in conjunction with each other. Any errors or omissions shall be the responsibility of the Architect before commencing work. Contractors are responsible to ensure requirements of the appropriate Building Code Authority, and all applicable laws and regulations are followed. All rights reserved.

DWG. NO.
SK1060

Scale:
1/32" = 1'-0"



Potential LB Units	4 Cluster Option	5 Cluster Option
Unit D: Single Storey Apt Net 1457.50 SF	24 Units 34,980 SF	30 Units 43,725 SF
Total Units Lower Bench - Net Total	24 Units 34,980 SF	30 Units 43,725 SF
Upper Bench - Gross Total Lower Bench - Gross Total	119,241.05 SF 154,221.05 SF	109,187 SF 152,912 SF
	FSR 0.74	FSR 0.74

Current Floor Area Ratio = 0.59	208,035.94 SQ FT 123,562.50 SQ FT	Permitted = 6.5 - 0.75
Site Area: Net Area:		
Current Floor Area Ratio with Circulation = 0.74	156,026.96 SF	
GIA with Circulation:	154,221.05 SQ FT	
Building Coverage Ratio =	?	
Building Coverage	?	
Existing Site Ratio = 0.57		
Existing Site Regulation:	? SQ FT	
Lot Coverage Ratio = 0.?		
Lot Coverage	? SQ FT	

Potential LB Units	4 Cluster Option	5 Cluster Option
Unit A: 3 Storey Townhouse L1 1200SF & L2 700SF	8 Units 15,200 SF	8 Units 15,200 SF
Unit A: Basement Studio Net 610 SF	8 Units 4,880 SF	8 Units 4,880 SF
Unit B: Single Storey Apt Net 1457.50 SF	16 Units 23,320 SF	16 Units 23,320 SF
Unit C: Single Storey Apt Net 1457.50 SF	24 Units 34,980 SF	24 Units 34,980 SF
Total Units Upper Bench - Net Total	56 Units 78,380 SF	56 Units 78,380 SF
Upper Bench - Gross Total	109,187 SF	

1st Row/Eaglecrest (10,540SF x4) = 42,160SF (GF 6982.5SF & L1 2247.5SF & L2 1310SF)	109,187 SF
2nd/Middle Row = (10,054SF x4) 40,216.2SF (3351.35SF x3 Levels)	
3rd Row = (10,054SF x4) = 28,810.80SF (3351.35SF x3 Levels)	

Potential LB Units	4 Cluster Option	5 Cluster Option
Unit A: 3 Storey Townhouse L1 1200SF & L2 700SF	8 Units 15,200 SF	8 Units 15,200 SF
Unit A: Basement Studio Net 610 SF	8 Units 4,880 SF	8 Units 4,880 SF
Unit B: Single Storey Apt Net 1457.50 SF	23 Units 33,522.5 SF	23 Units 33,522.5 SF
Unit C: Single Storey Apt Net 1457.50 SF	24 Units 34,980 SF	24 Units 34,980 SF
Total Units Upper Bench - Net Total	63 Units 88,582.5 SF	63 Units 88,582.5 SF
Upper Bench - Gross Total	119,241.05 SF	

1st Row/Eaglecrest (10,540SF x4) = 42,160SF (GF 6982.5SF & L1 2247.5SF & L2 1310SF)	119,241.05 SF
2nd/Middle Row = (10,054SF x4) 40,216.2SF (3351.35SF x3 Levels)	
3rd Row = (10,054SF x4) - 1 Unit = 36,864.85SF (3351.35SF x3 Levels)	

SITE PLAN - New Cluster Massing - Inlets Meets Grade Level
1/32" = 1'-0"

APPENDIX B

Traffic Movement Count Summary Sheets



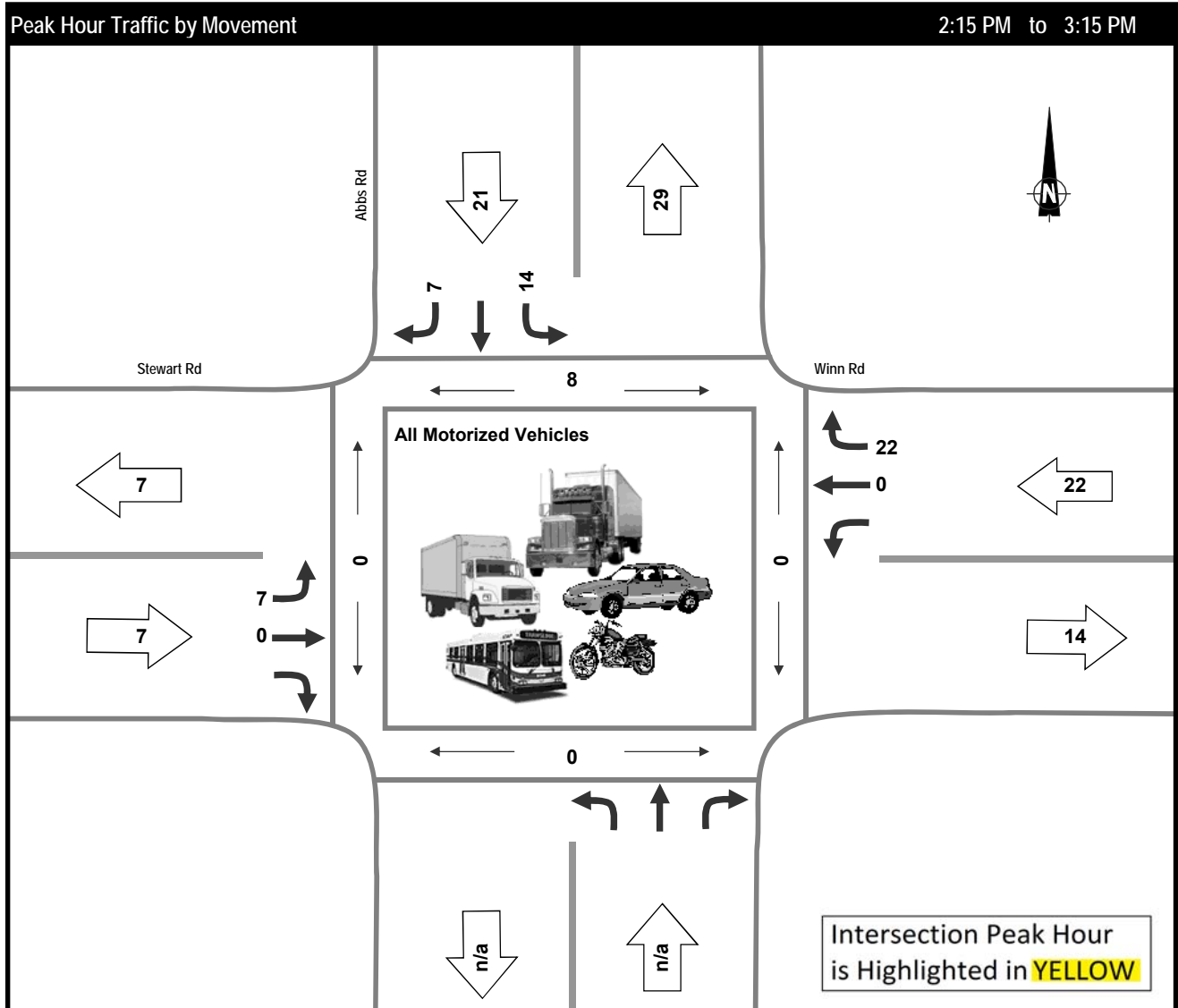
Vehicle Classification Summary

Project: #5602: 464 Eaglecrest Drive - Traffic Engineering Services
Municipality: Gibsons, BC
Weather: Clear, Sunny

Time Period	Entering Intersection	Vehicle Classification				Total
		Passenger Cars	Heavy Vehicles (3 or more axles)			
Morning	Volume					
	%					
Midday	Volume					
	%					
Afternoon (14:00 - 17:30)	Volume	138	0			138
	%	100.0%	0.0%			100.0%
Total (3.5 Hours)	Volume	138	0			138
	%	100.0%	0.0%			100.0%

Project: #5602: 464 Eaglecrest Drive - Traffic Engineering Services
 Municipality: Gibsons, BC
 Weather: Clear, Sunny
 Vehicle Class: All Motorized Vehicles

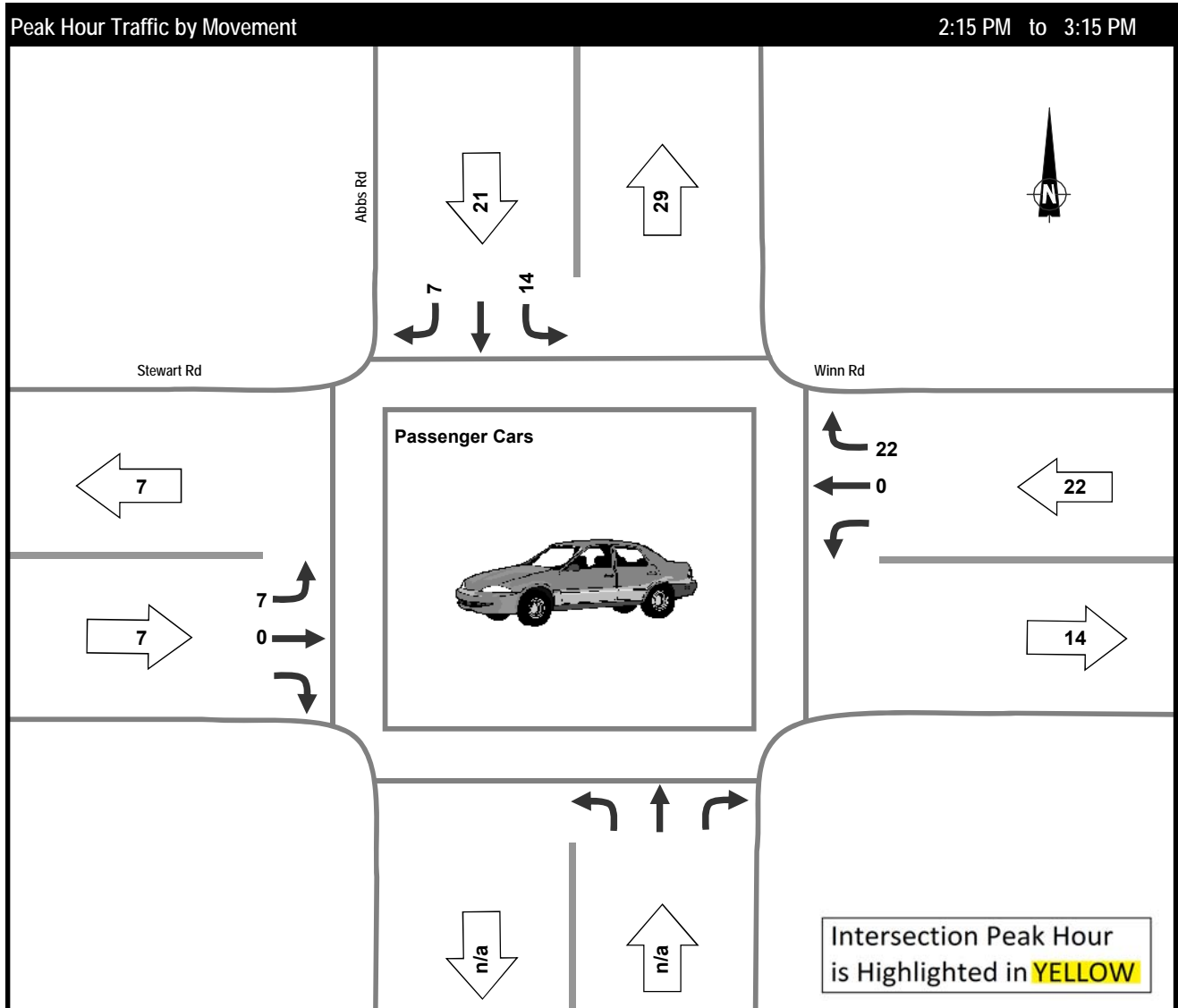
Afternoon Peak Period



Time	NORTH Approach			SOUTH Approach			WEST Approach			EAST Approach			PEDESTRIANS				Total Volumes
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	N	S	W	E	
Peak Hour	14		7				7	0			0	22	8	0	0	0	50
PHF	0.70		0.58				0.58	0.00			0.00	0.61	0.33	0.00	0.00	0.00	0.78
Peak 15 X 4	20		12				12	0			0	36	24	0	0	0	64
Average Hour	11		6				7	1			1	15	5	0	1	0	41
Survey Total	38		21				23	2			3	51	17	0	5	1	138
14:00	1		3				3	0			0	2	0	0	0	0	9
14:15	2		2				1	0			0	3	2	0	0	0	8
14:30	4		2				3	0			0	6	0	0	0	0	15
14:45	3		3				1	0			0	4	0	0	0	0	11
15:00	5		0				2	0			0	9	6	0	0	0	16
15:15	1		1				1	0			0	1	1	0	0	0	4
15:30	2		0				4	0			0	2	2	0	0	0	8
15:45	1		2				1	0			1	2	1	0	0	0	7
16:00	4		0				1	0			0	5	0	0	2	0	10
16:15	3		0				1	0			1	3	0	0	2	0	8
16:30	1		2				2	0			0	3	2	0	0	1	8
16:45	3		2				1	2			1	3	1	0	0	0	12
17:00	5		2				0	0			0	5	1	0	0	0	12
17:15	3		2				2	0			0	3	1	0	1	0	10

Project: #5602: 464 Eaglecrest Drive - Traffic Engineering Services
 Municipality: Gibsons, BC
 Weather: Clear, Sunny
 Vehicle Class: Passenger Cars

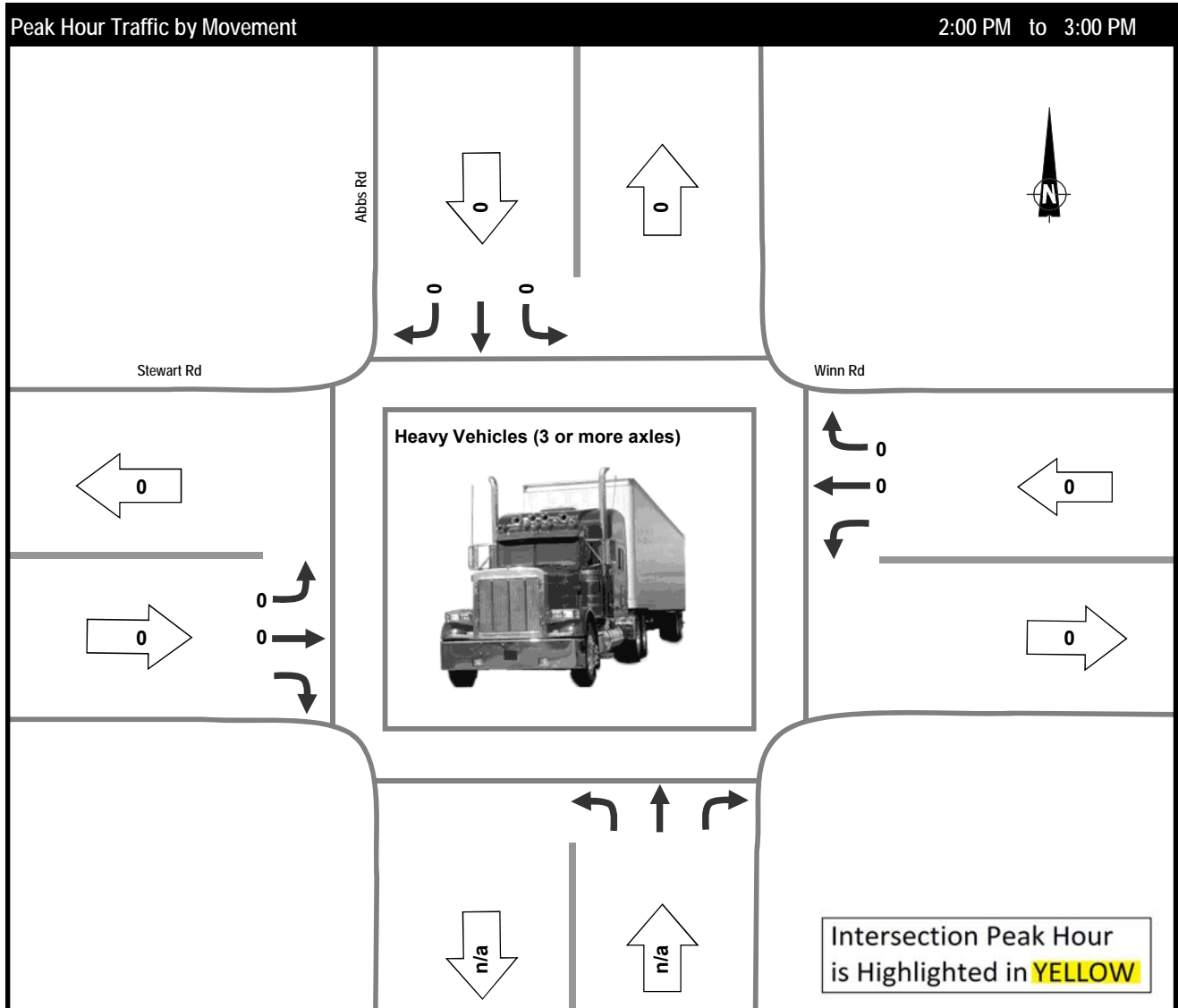
Afternoon Peak Period



Time	NORTH Approach			SOUTH Approach			WEST Approach			EAST Approach			PEDESTRIANS				Total Volumes
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	N	S	W	E	
Peak Hour	14		7				7	0			0	22					50
PHF	0.70		0.58				0.58	0.00			0.00	0.61					0.78
Peak 15 X 4	20		12				12	0			0	36					64
Average Hour	11		6				7	1			1	15					41
Survey Total	38		21				23	2			3	51					138
14:00	1		3				3	0			0	2					9
14:15	2		2				1	0			0	3					8
14:30	4		2				3	0			0	6					15
14:45	3		3				1	0			0	4					11
15:00	5		0				2	0			0	9					16
15:15	1		1				1	0			0	1					4
15:30	2		0				4	0			0	2					8
15:45	1		2				1	0			1	2					7
16:00	4		0				1	0			0	5					10
16:15	3		0				1	0			1	3					8
16:30	1		2				2	0			0	3					8
16:45	3		2				1	2			1	3					12
17:00	5		2				0	0			0	5					12
17:15	3		2				2	0			0	3					10

Project: #5602: 464 Eaglecrest Drive - Traffic Engineering Services
 Municipality: Gibsons, BC
 Weather: Clear, Sunny
 Vehicle Class: Heavy Vehicles (3 or more axles)

Afternoon Peak Period

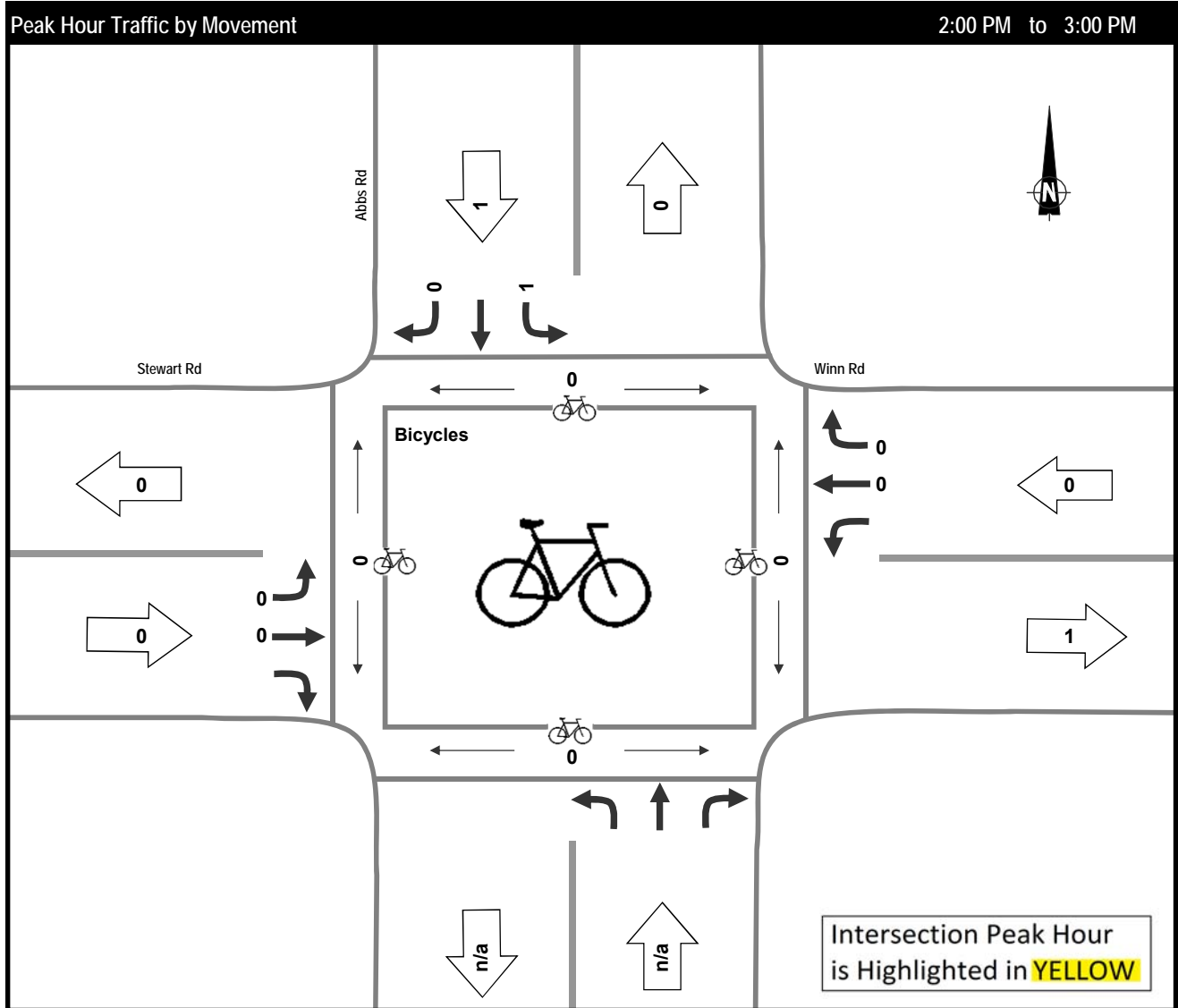


Time	NORTH Approach			SOUTH Approach			WEST Approach			EAST Approach			PEDESTRIANS				Total Volumes
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	N	S	W	E	
Peak Hour	0		0				0	0			0	0					0
PHF	0.00		0.00				0.00	0.00			0.00	0.00					0.00
Peak 15 X 4	0		0				0	0			0	0					0
Average Hour	0		0				0	0			0	0					0
Survey Total	0		0				0	0			0	0					0
14:00	0		0				0	0			0	0					0
14:15	0		0				0	0			0	0					0
14:30	0		0				0	0			0	0					0
14:45	0		0				0	0			0	0					0
15:00	0		0				0	0			0	0					0
15:15	0		0				0	0			0	0					0
15:30	0		0				0	0			0	0					0
15:45	0		0				0	0			0	0					0
16:00	0		0				0	0			0	0					0
16:15	0		0				0	0			0	0					0
16:30	0		0				0	0			0	0					0
16:45	0		0				0	0			0	0					0
17:00	0		0				0	0			0	0					0
17:15	0		0				0	0			0	0					0

Afternoon Peak Period

Project: #5602: 464 Eaglecrest Drive - Traffic Engineering Services
 Municipality: Gibsons, BC
 Weather: Clear, Sunny
 Vehicle Class: Bicycles

Note: Crosswalk bike volumes shown are cyclists who rode their bike across the crosswalk and are not included in the pedestrian volume totals



Time	NORTH Approach			SOUTH Approach			WEST Approach			EAST Approach			BIKES in X-WALKS				Total Volumes
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	N	S	W	E	
Peak Hour	1		0				0	0			0	0	0	0	0	0	1
PHF	0.25		0.00				0.00	0.00			0.00	0.00	0.00	0.00	0.00	0.00	0.25
Peak 15 X 4	4		0				0	0			0	0	0	0	0	0	4
Average Hour	0		0				0	0			0	0	0	0	0	0	0
Survey Total	1		0				0	0			1	0	0	0	0	0	2
14:00	0		0				0	0			0	0	0	0	0	0	0
14:15	0		0				0	0			0	0	0	0	0	0	0
14:30	1		0				0	0			0	0	0	0	0	0	1
14:45	0		0				0	0			0	0	0	0	0	0	0
15:00	0		0				0	0			0	0	0	0	0	0	0
15:15	0		0				0	0			0	0	0	0	0	0	0
15:30	0		0				0	0			0	0	0	0	0	0	0
15:45	0		0				0	0			0	0	0	0	0	0	0
16:00	0		0				0	0			0	0	0	0	0	0	0
16:15	0		0				0	0			0	0	0	0	0	0	0
16:30	0		0				0	0			0	0	0	0	0	0	0
16:45	0		0				0	0			0	0	0	0	0	0	0
17:00	0		0				0	0			1	0	0	0	0	0	1
17:15	0		0				0	0			0	0	0	0	0	0	0



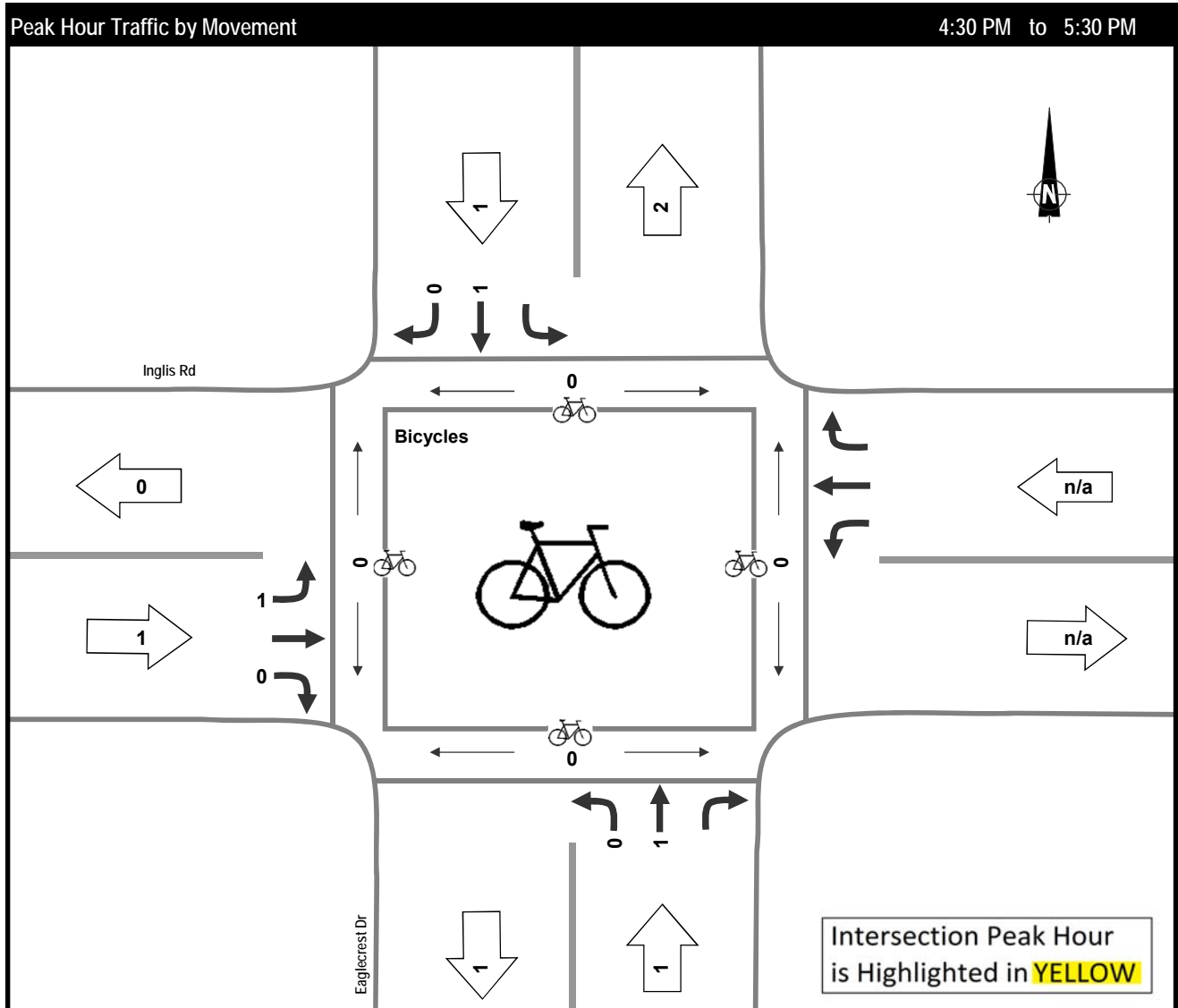
Vehicle Classification Summary

Project: #5602: 464 Eaglecrest Drive - Traffic Engineering Services
Municipality: Gibsons, BC
Weather: Sunny

Time Period	Entering Intersection	Vehicle Classification				Total
		Passenger Cars	Heavy Vehicles (3 or more axles)			
Morning	Volume					
	%					
Midday	Volume					
	%					
Afternoon (14:00 - 17:30)	Volume	26	0			26
	%	100.0%	0.0%			100.0%
Total (3.5 Hours)	Volume	26	0			26
	%	100.0%	0.0%			100.0%

Project: #5602: 464 Eaglecrest Drive - Traffic Engineering Services
Municipality: Gibsons, BC
Weather: Sunny
Vehicle Class: Bicycles
Note: Crosswalk bike volumes shown are cyclists who rode their bike across the crosswalk and are not included in the pedestrian volume totals

Afternoon Peak Period



Time	NORTH Approach			SOUTH Approach			WEST Approach			EAST Approach			BIKES in X-WALKS				Total Volumes
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	N	S	W	E	
Peak Hour		1	0	0	1		1		0				0	0	0	0	3
PHF		0.25	0.00	0.00	0.25		0.25		0.00				0.00	0.00	0.00	0.00	0.25
Peak 15 X 4		4	0	0	4		4		0				0	0	0	0	12
Average Hour		0	0	0	1		1		0				0	0	0	0	2
Survey Total		1	0	0	2		2		0				0	0	0	0	5
14:00		0	0	0	0		0		0				0	0	0	0	0
14:15		0	0	0	0		0		0				0	0	0	0	0
14:30		0	0	0	0		0		0				0	0	0	0	0
14:45		0	0	0	0		0		0				0	0	0	0	0
15:00		0	0	0	0		0		0				0	0	0	0	0
15:15		0	0	0	1		0		0				0	0	0	0	1
15:30		0	0	0	0		0		0				0	0	0	0	0
15:45		0	0	0	0		1		0				0	0	0	0	1
16:00		0	0	0	0		0		0				0	0	0	0	0
16:15		0	0	0	0		0		0				0	0	0	0	0
16:30		0	0	0	0		0		0				0	0	0	0	0
16:45		0	0	0	0		0		0				0	0	0	0	0
17:00		0	0	0	0		0		0				0	0	0	0	0
17:15		1	0	0	1		1		0				0	0	0	0	3



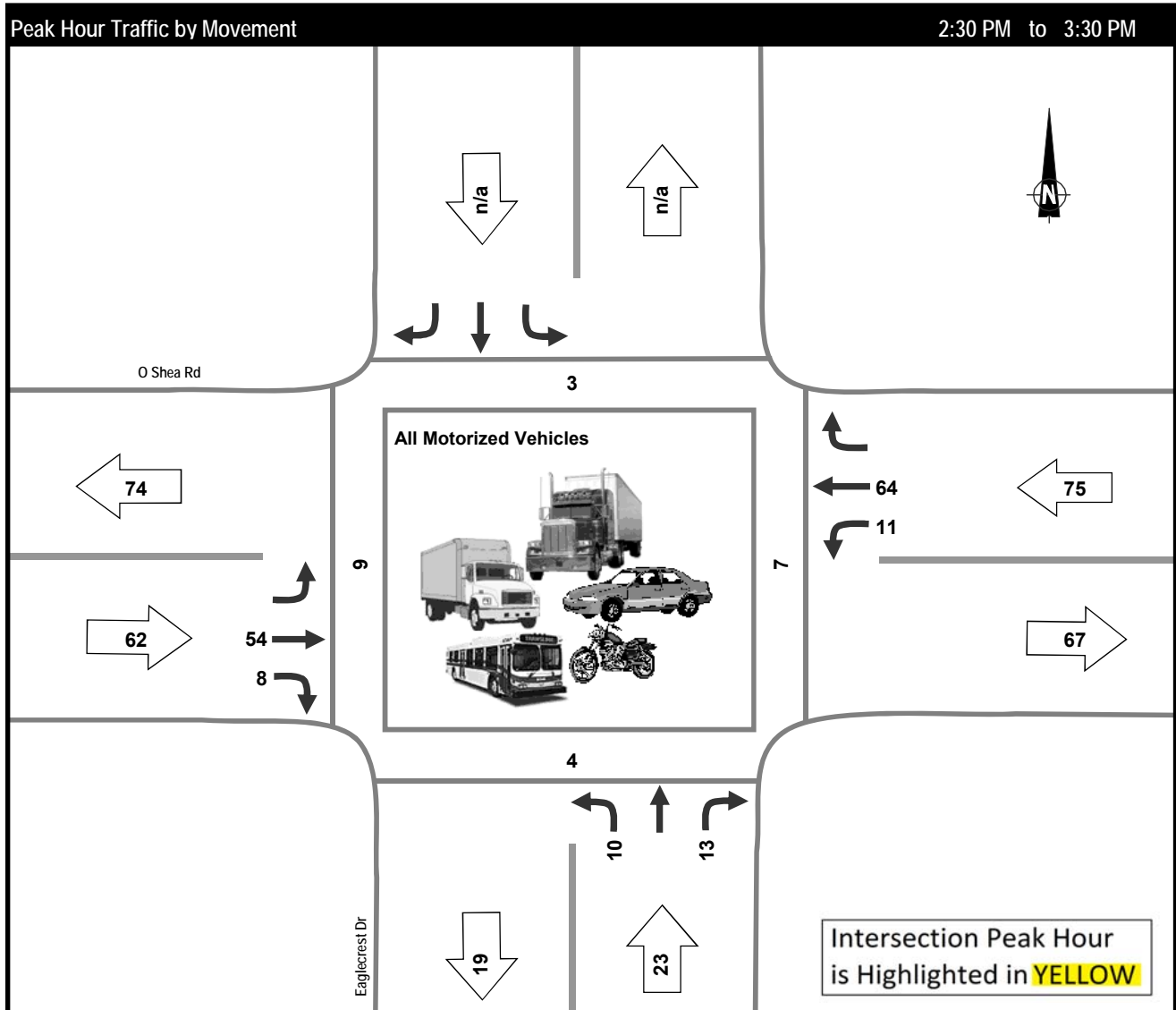
Vehicle Classification Summary

Project: #5602: 464 Eaglecrest Drive - Traffic Engineering Services
Municipality: Gibsons, BC
Weather: Sunny

Time Period	Entering Intersection	Vehicle Classification				Total
		Passenger Cars	Heavy Vehicles (3 or more axles)			
Morning	Volume					
	%					
Midday	Volume					
	%					
Afternoon (14:00 - 17:30)	Volume	420	0			420
	%	100.0%	0.0%			100.0%
Total (3.5 Hours)	Volume	420	0			420
	%	100.0%	0.0%			100.0%

Project: #5602: 464 Eaglecrest Drive - Traffic Engineering Services
Municipality: Gibsons, BC
Weather: Sunny
Vehicle Class: All Motorized Vehicles

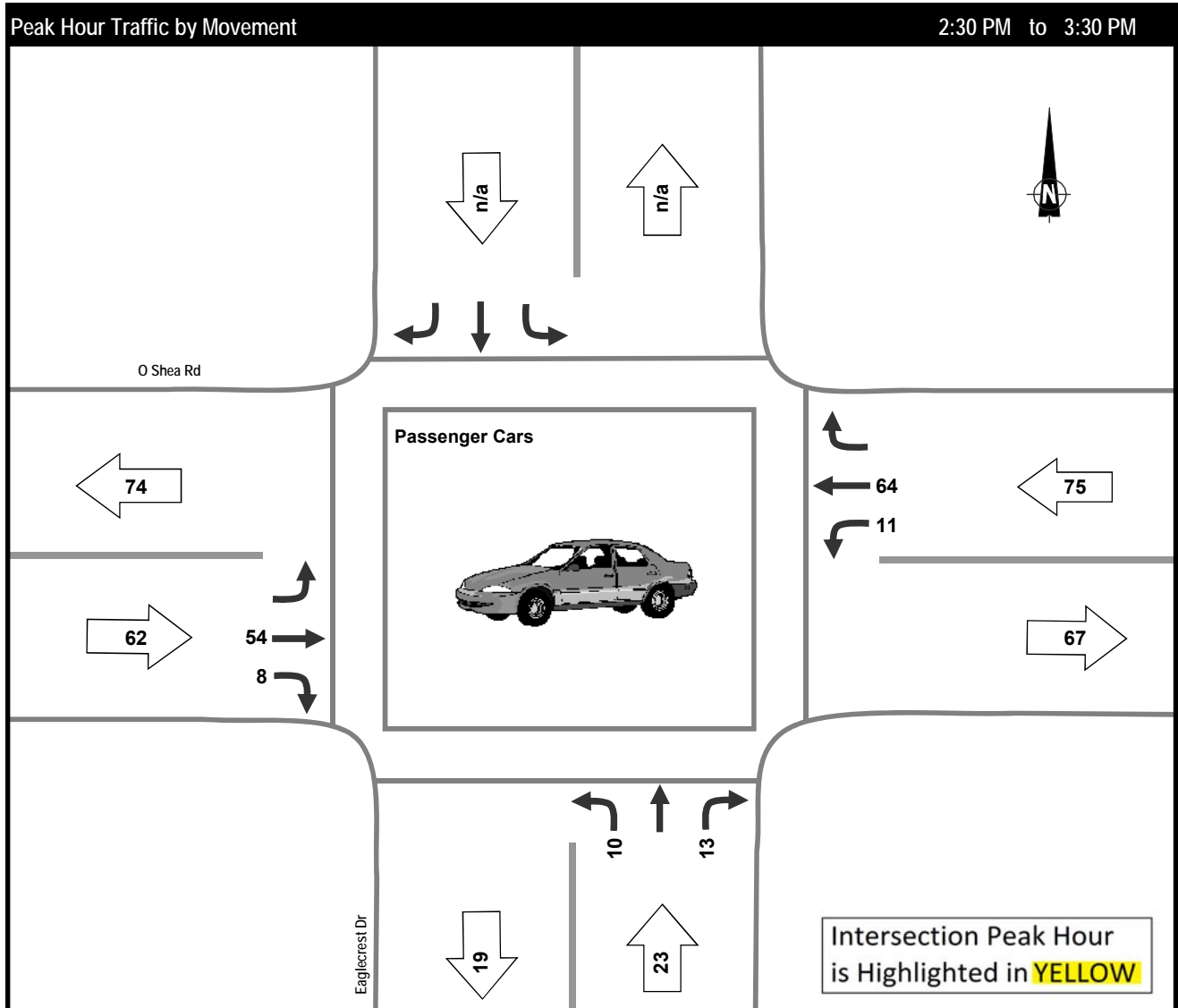
Afternoon Peak Period



Time	NORTH Approach			SOUTH Approach			WEST Approach			EAST Approach			PEDESTRIANS				Total Volumes
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	N	S	W	E	
Peak Hour				10		13	54	8	11	64			3	4	9	7	160
PHF				0.63		0.41	0.75	0.67	0.69	0.76			0.75	0.50	0.56	0.35	0.82
Peak 15 X 4				16		32	72	12	16	84			4	8	16	20	196
Average Hour				11		13	40	8	11	38			6	3	5	5	121
Survey Total				37		44	139	28	38	134			21	9	19	18	420
14:00				5		5	10	1	2	8			2	1	0	2	31
14:15				4		3	11	2	3	6			1	3	4	2	29
14:30				0		2	9	2	4	10			1	0	0	0	27
14:45				3		8	18	1	0	19			1	0	4	5	49
15:00				3		2	15	3	4	21			1	2	1	2	48
15:15				4		1	12	2	3	14			0	2	4	0	36
15:30				2		4	7	4	3	5			3	0	1	1	25
15:45				1		2	10	2	1	9			2	0	0	0	25
16:00				1		5	9	4	1	11			2	0	0	0	31
16:15				0		0	7	1	4	4			3	0	2	0	16
16:30				5		3	11	3	0	5			0	1	0	2	27
16:45				3		4	9	0	4	9			5	0	0	3	29
17:00				0		1	5	2	5	7			0	0	0	0	20
17:15				6		4	6	1	4	6			0	0	3	1	27

Project: #5602: 464 Eaglecrest Drive - Traffic Engineering Services
 Municipality: Gibsons, BC
 Weather: Sunny
 Vehicle Class: Passenger Cars

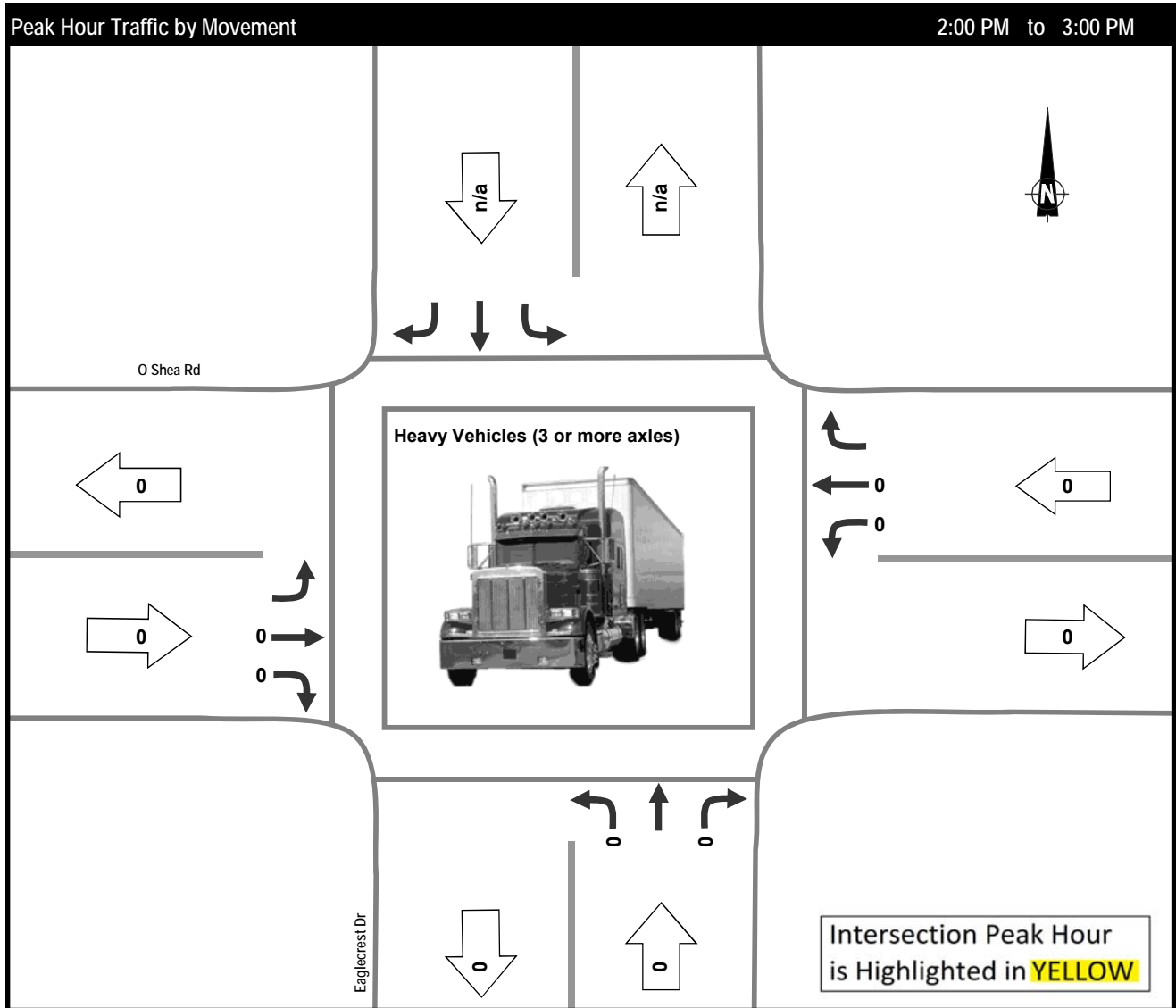
Afternoon Peak Period



Time	NORTH Approach			SOUTH Approach			WEST Approach			EAST Approach			PEDESTRIANS				Total Volumes
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	N	S	W	E	
Peak Hour				10		13	54	8		11	64						160
PHF				0.63		0.41	0.75	0.67		0.69	0.76						0.82
Peak 15 X 4				16		32	72	12		16	84						196
Average Hour				11		13	40	8		11	38						121
Survey Total				37		44	139	28		38	134						420
14:00				5		5	10	1		2	8						31
14:15				4		3	11	2		3	6						29
14:30				0		2	9	2		4	10						27
14:45				3		8	18	1		0	19						49
15:00				3		2	15	3		4	21						48
15:15				4		1	12	2		3	14						36
15:30				2		4	7	4		3	5						25
15:45				1		2	10	2		1	9						25
16:00				1		5	9	4		1	11						31
16:15				0		0	7	1		4	4						16
16:30				5		3	11	3		0	5						27
16:45				3		4	9	0		4	9						29
17:00				0		1	5	2		5	7						20
17:15				6		4	6	1		4	6						27

Project: #5602: 464 Eaglecrest Drive - Traffic Engineering Services
 Municipality: Gibsons, BC
 Weather: Sunny
 Vehicle Class: Heavy Vehicles (3 or more axles)

Afternoon Peak Period

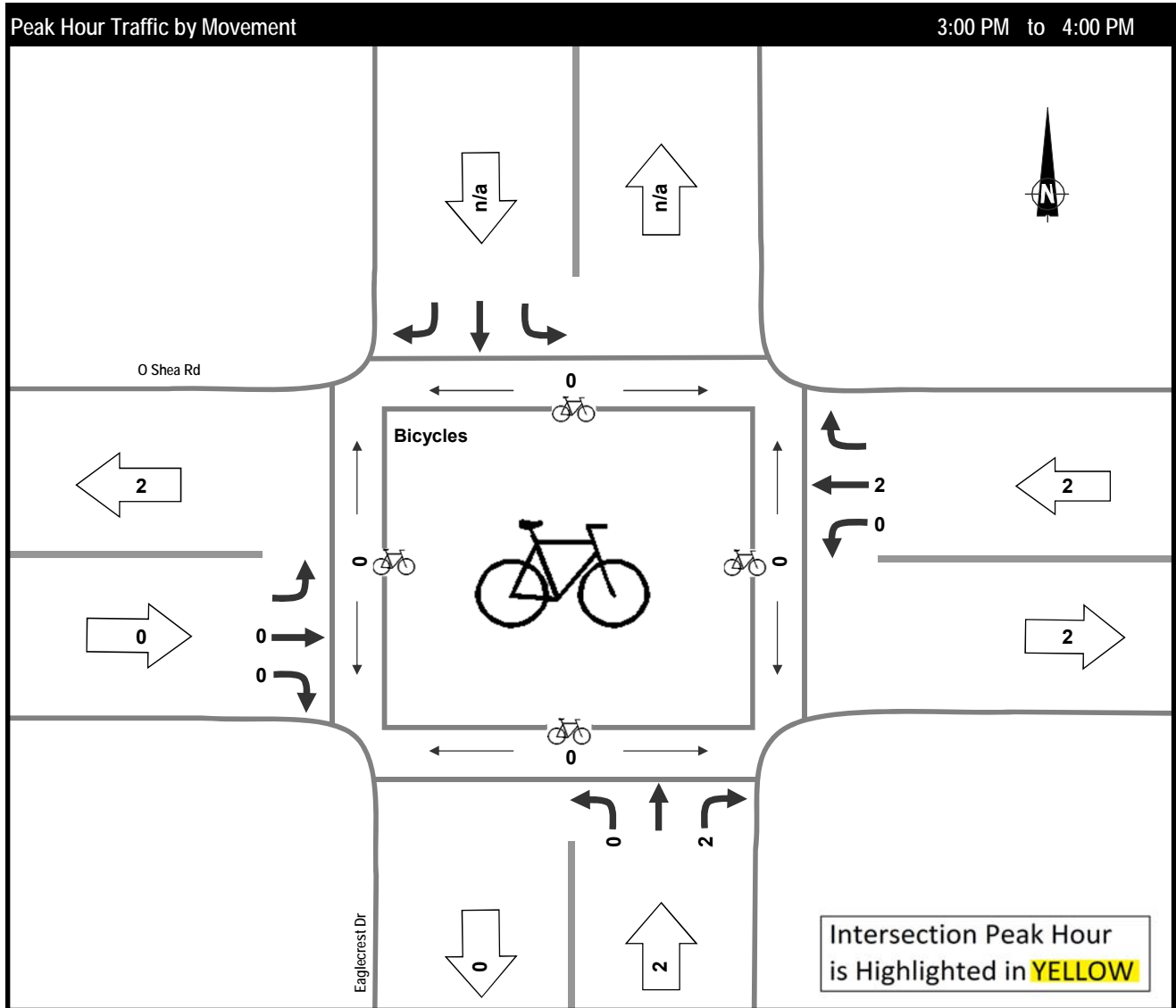


Time	NORTH Approach			SOUTH Approach			WEST Approach			EAST Approach			PEDESTRIANS				Total Volumes
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	N	S	W	E	
Peak Hour				0		0	0	0	0	0	0						0
PHF				0.00		0.00	0.00	0.00	0.00	0.00	0.00						0.00
Peak 15 X 4				0		0	0	0	0	0	0						0
Average Hour				0		0	0	0	0	0	0						0
Survey Total				0		0	0	0	0	0	0						0
14:00				0		0	0	0	0	0	0						0
14:15				0		0	0	0	0	0	0						0
14:30				0		0	0	0	0	0	0						0
14:45				0		0	0	0	0	0	0						0
15:00				0		0	0	0	0	0	0						0
15:15				0		0	0	0	0	0	0						0
15:30				0		0	0	0	0	0	0						0
15:45				0		0	0	0	0	0	0						0
16:00				0		0	0	0	0	0	0						0
16:15				0		0	0	0	0	0	0						0
16:30				0		0	0	0	0	0	0						0
16:45				0		0	0	0	0	0	0						0
17:00				0		0	0	0	0	0	0						0
17:15				0		0	0	0	0	0	0						0

Project: #5602: 464 Eaglecrest Drive - Traffic Engineering Services
 Municipality: Gibsons, BC
 Weather: Sunny
 Vehicle Class: Bicycles

Afternoon Peak Period

Note: Crosswalk bike volumes shown are cyclists who rode their bike across the crosswalk and are not included in the pedestrian volume totals



Time	NORTH Approach			SOUTH Approach			WEST Approach			EAST Approach			BIKES in X-WALKS				Total Volumes
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	N	S	W	E	
Peak Hour				0		2		0	0	0	2		0	0	0	0	4
PHF				0.00		0.50		0.00	0.00	0.00	0.50		0.00	0.00	0.00	0.00	0.50
Peak 15 X 4				0		4		0	0	0	4		0	0	0	0	8
Average Hour				0		1		0	0	0	1		0	0	0	0	2
Survey Total				0		2		0	0	0	2		0	0	0	0	4
14:00				0		0		0	0	0	0		0	0	0	0	0
14:15				0		0		0	0	0	0		0	0	0	0	0
14:30				0		0		0	0	0	0		0	0	0	0	0
14:45				0		0		0	0	0	0		0	0	0	0	0
15:00				0		0		0	0	0	0		0	0	0	0	0
15:15				0		1		0	0	0	1		0	0	0	0	2
15:30				0		0		0	0	0	0		0	0	0	0	0
15:45				0		1		0	0	0	1		0	0	0	0	2
16:00				0		0		0	0	0	0		0	0	0	0	0
16:15				0		0		0	0	0	0		0	0	0	0	0
16:30				0		0		0	0	0	0		0	0	0	0	0
16:45				0		0		0	0	0	0		0	0	0	0	0
17:00				0		0		0	0	0	0		0	0	0	0	0
17:15				0		0		0	0	0	0		0	0	0	0	0



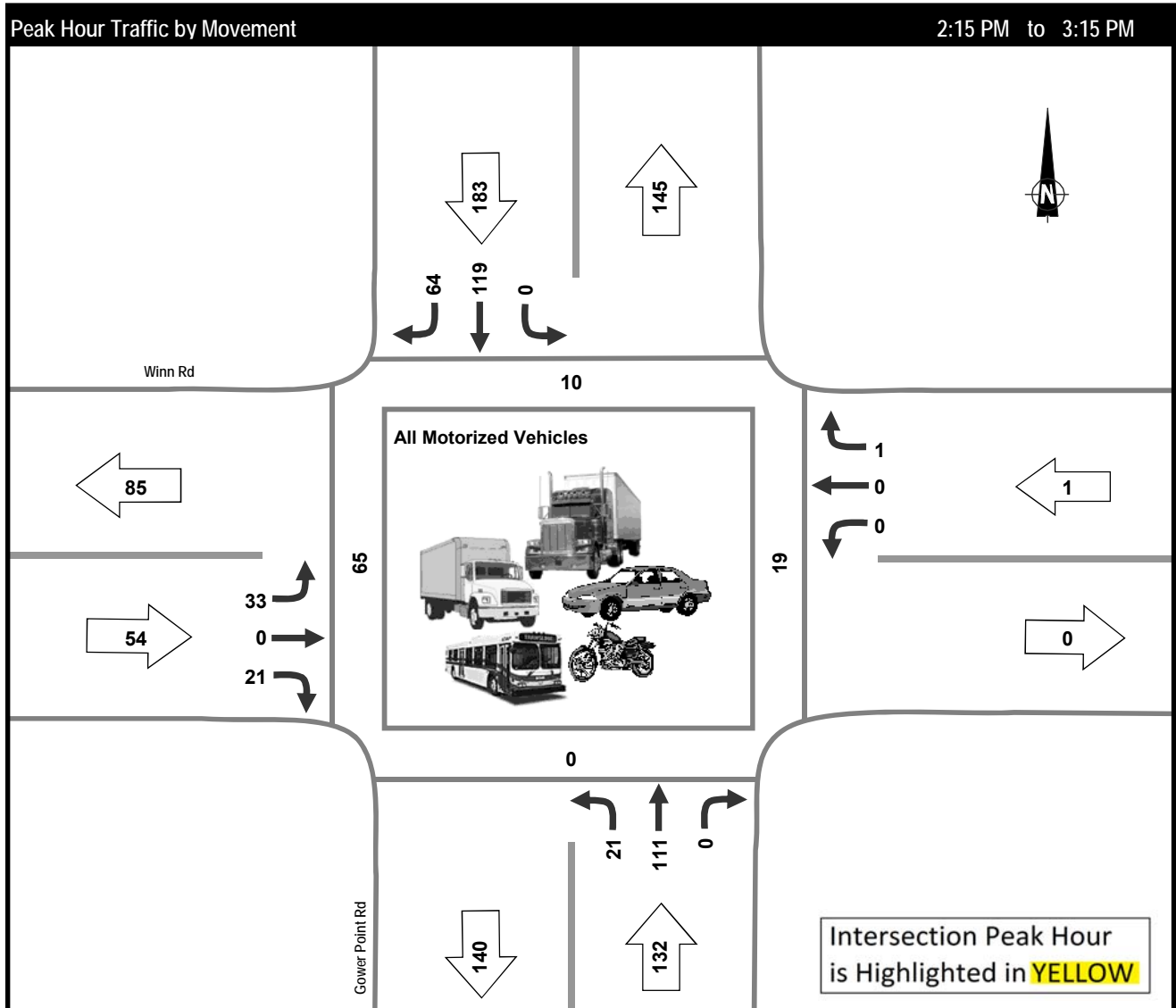
Vehicle Classification Summary

Project: #5602: 464 Eaglecrest Drive - Traffic Engineering Services
Municipality: Gibsons, BC
Weather: Sunny

Time Period	Entering Intersection	Vehicle Classification				Total
		Passenger Cars	Heavy Vehicles (3 or more axles)			
Morning	Volume					
	%					
Midday	Volume					
	%					
Afternoon (14:00 - 17:30)	Volume	1,152	12			1,164
	%	99.0%	1.0%			100.0%
Total (3.5 Hours)	Volume	1,152	12			1,164
	%	99.0%	1.0%			100.0%

Project: #5602: 464 Eaglecrest Drive - Traffic Engineering Services
Municipality: Gibsons, BC
Weather: Sunny
Vehicle Class: All Motorized Vehicles

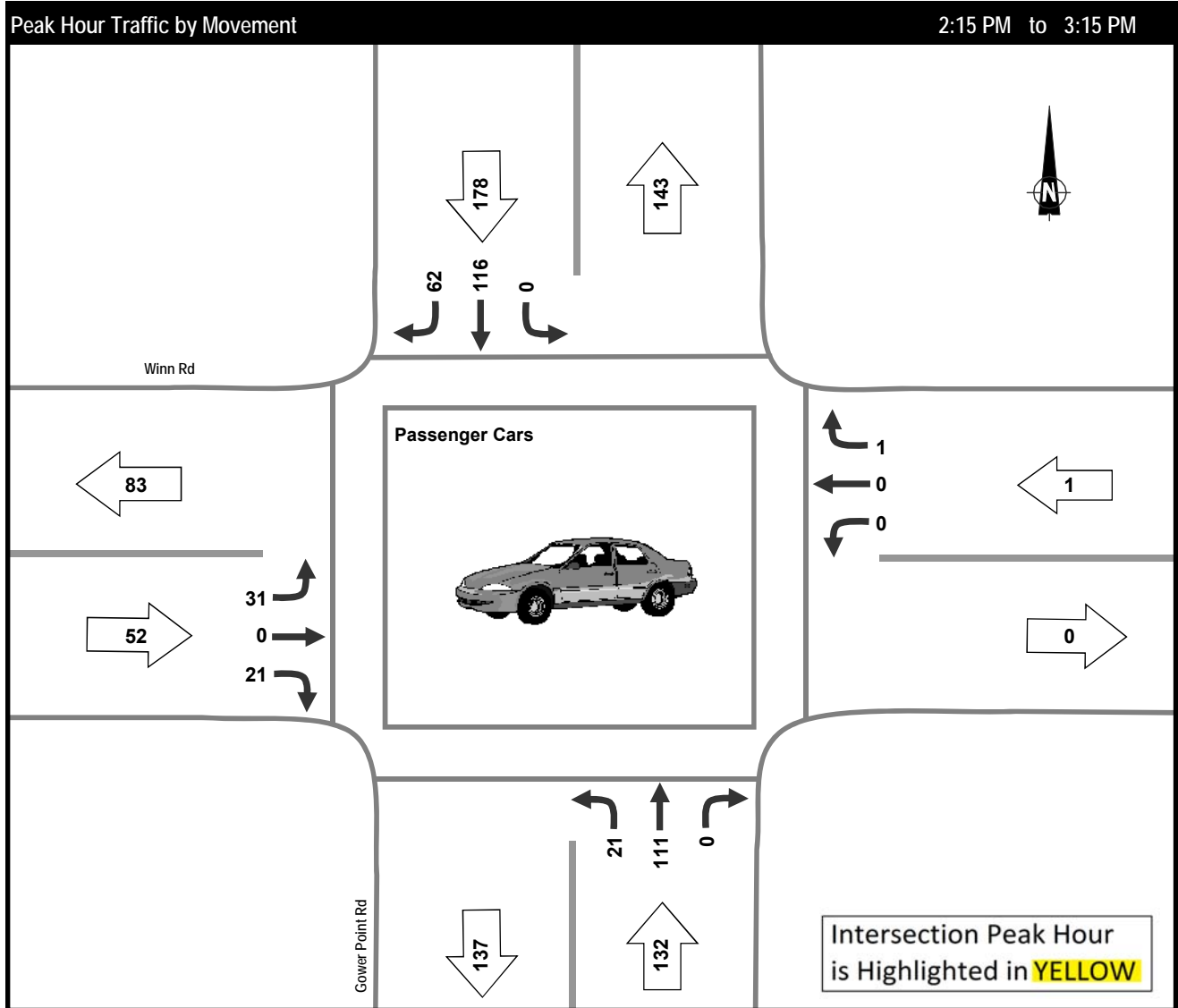
Afternoon Peak Period



Time	NORTH Approach			SOUTH Approach			WEST Approach			EAST Approach			PEDESTRIANS				Total Volumes
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	N	S	W	E	
Peak Hour	0	119	64	21	111	0	33	0	21	0	0	1	10	0	65	19	370
PHF	0.00	0.80	0.70	0.88	0.90	0.00	0.92	0.00	0.58	0.00	0.00	0.25	0.42	0.00	0.63	0.30	0.89
Peak 15 X 4	0	148	92	24	124	0	36	0	36	0	0	4	24	0	104	64	416
Average Hour	0	115	44	20	106	0	30	0	17	0	0	0	14	2	68	28	332
Survey Total	0	403	153	71	370	1	105	0	59	1	0	1	50	6	237	97	1,164
14:00	0	20	11	6	18	0	6	0	3	0	0	0	14	1	34	36	64
14:15	0	31	16	5	25	0	9	0	1	0	0	0	4	0	26	1	87
14:30	0	20	15	6	30	0	9	0	9	0	0	0	0	0	10	1	89
14:45	0	37	23	5	25	0	9	0	5	0	0	0	0	0	9	1	104
15:00	0	31	10	5	31	0	6	0	6	0	0	1	6	0	20	16	90
15:15	0	25	7	3	29	1	9	0	2	0	0	0	4	0	23	5	76
15:30	0	28	9	6	34	0	13	0	8	0	0	0	7	0	34	3	98
15:45	0	33	13	5	32	0	3	0	3	0	0	0	2	1	16	5	89
16:00	0	17	8	8	29	0	10	0	4	0	0	0	2	2	13	7	76
16:15	0	32	11	5	32	0	7	0	0	1	0	0	1	0	12	7	88
16:30	0	21	9	5	26	0	8	0	7	0	0	0	2	1	13	6	76
16:45	0	25	4	7	29	0	9	0	3	0	0	0	4	1	9	3	77
17:00	0	48	12	3	13	0	4	0	4	0	0	0	1	0	12	0	84
17:15	0	35	5	2	17	0	3	0	4	0	0	0	3	0	6	6	66

Project: #5602: 464 Eaglecrest Drive - Traffic Engineering Services
 Municipality: Gibsons, BC
 Weather: Sunny
 Vehicle Class: Passenger Cars

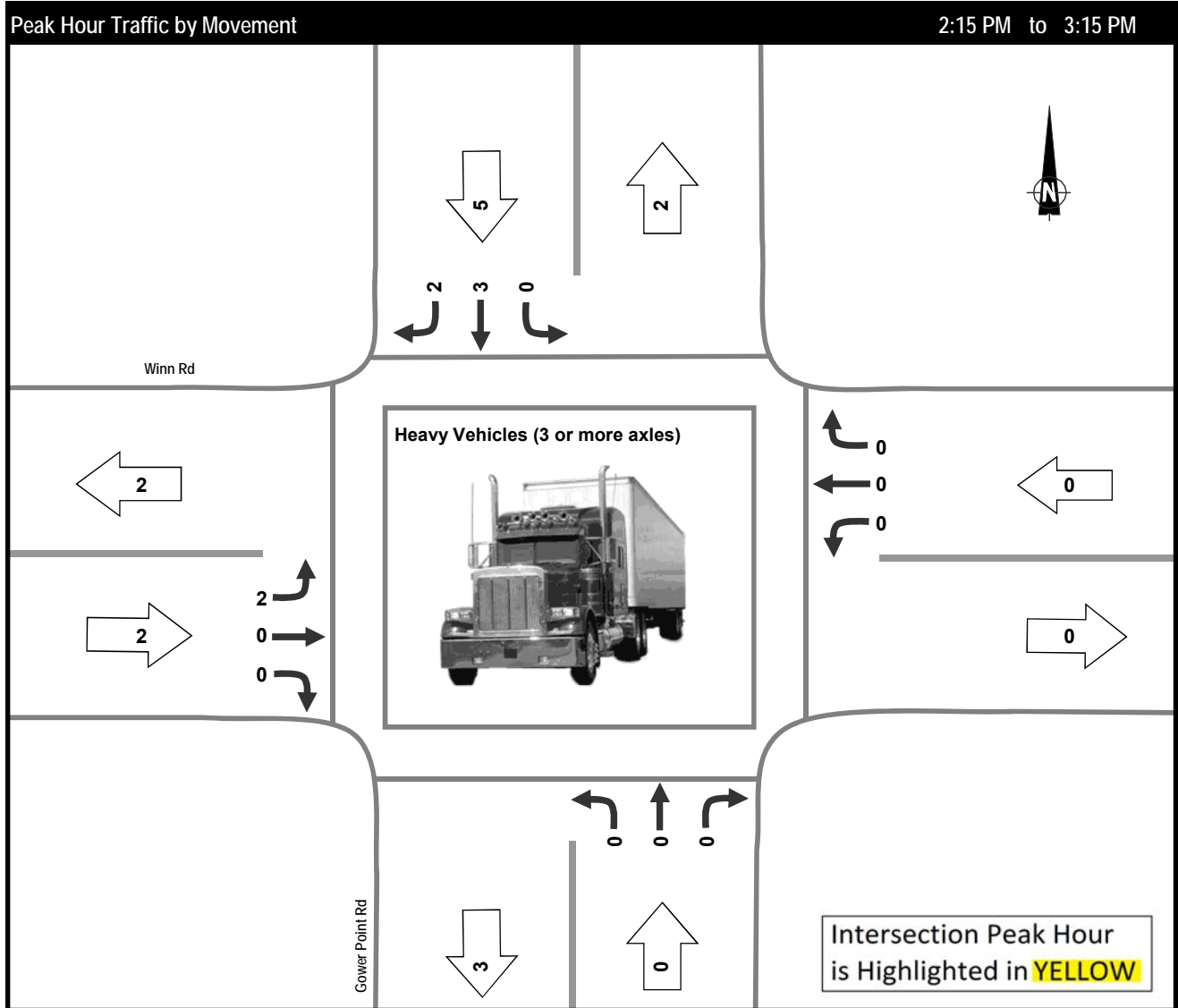
Afternoon Peak Period



Time	NORTH Approach			SOUTH Approach			WEST Approach			EAST Approach			PEDESTRIANS				Total Volumes
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	N	S	W	E	
Peak Hour	0	116	62	21	111	0	31	0	21	0	0	1					363
PHF	0.00	0.81	0.70	0.88	0.90	0.00	0.86	0.00	0.58	0.00	0.00	0.25					0.89
Peak 15 X 4	0	144	88	24	124	0	36	0	36	0	0	4					408
Average Hour	0	113	43	20	106	0	29	0	17	0	0	0					328
Survey Total	0	397	150	71	370	1	102	0	59	1	0	1					1,152
14:00	0	19	11	6	18	0	6	0	3	0	0	0					63
14:15	0	31	15	5	25	0	9	0	1	0	0	0					86
14:30	0	19	15	6	30	0	8	0	9	0	0	0					87
14:45	0	36	22	5	25	0	9	0	5	0	0	0					102
15:00	0	30	10	5	31	0	5	0	6	0	0	1					88
15:15	0	25	6	3	29	1	9	0	2	0	0	0					75
15:30	0	27	9	6	34	0	12	0	8	0	0	0					96
15:45	0	32	13	5	32	0	3	0	3	0	0	0					88
16:00	0	17	8	8	29	0	10	0	4	0	0	0					76
16:15	0	32	11	5	32	0	7	0	0	1	0	0					88
16:30	0	21	9	5	26	0	8	0	7	0	0	0					76
16:45	0	25	4	7	29	0	9	0	3	0	0	0					77
17:00	0	48	12	3	13	0	4	0	4	0	0	0					84
17:15	0	35	5	2	17	0	3	0	4	0	0	0					66

Project: #5602: 464 Eaglecrest Drive - Traffic Engineering Services
Municipality: Gibsons, BC
Weather: Sunny
Vehicle Class: Heavy Vehicles (3 or more axles)

Afternoon Peak Period



Time	NORTH Approach			SOUTH Approach			WEST Approach			EAST Approach			PEDESTRIANS				Total Volumes
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	N	S	W	E	
Peak Hour	0	3	2	0	0	0	2	0	0	0	0	0					7
PHF	0.00	0.75	0.50	0.00	0.00	0.00	0.50	0.00	0.00	0.00	0.00	0.00					0.88
Peak 15 X 4	0	4	4	0	0	0	4	0	0	0	0	0					8
Average Hour	0	2	1	0	0	0	1	0	0	0	0	0					4
Survey Total	0	6	3	0	0	0	3	0	0	0	0	0					12
14:00	0	1	0	0	0	0	0	0	0	0	0	0					1
14:15	0	0	1	0	0	0	0	0	0	0	0	0					1
14:30	0	1	0	0	0	0	1	0	0	0	0	0					2
14:45	0	1	1	0	0	0	0	0	0	0	0	0					2
15:00	0	1	0	0	0	0	1	0	0	0	0	0					2
15:15	0	0	1	0	0	0	0	0	0	0	0	0					1
15:30	0	1	0	0	0	0	1	0	0	0	0	0					2
15:45	0	1	0	0	0	0	0	0	0	0	0	0					1
16:00	0	0	0	0	0	0	0	0	0	0	0	0					0
16:15	0	0	0	0	0	0	0	0	0	0	0	0					0
16:30	0	0	0	0	0	0	0	0	0	0	0	0					0
16:45	0	0	0	0	0	0	0	0	0	0	0	0					0
17:00	0	0	0	0	0	0	0	0	0	0	0	0					0
17:15	0	0	0	0	0	0	0	0	0	0	0	0					0



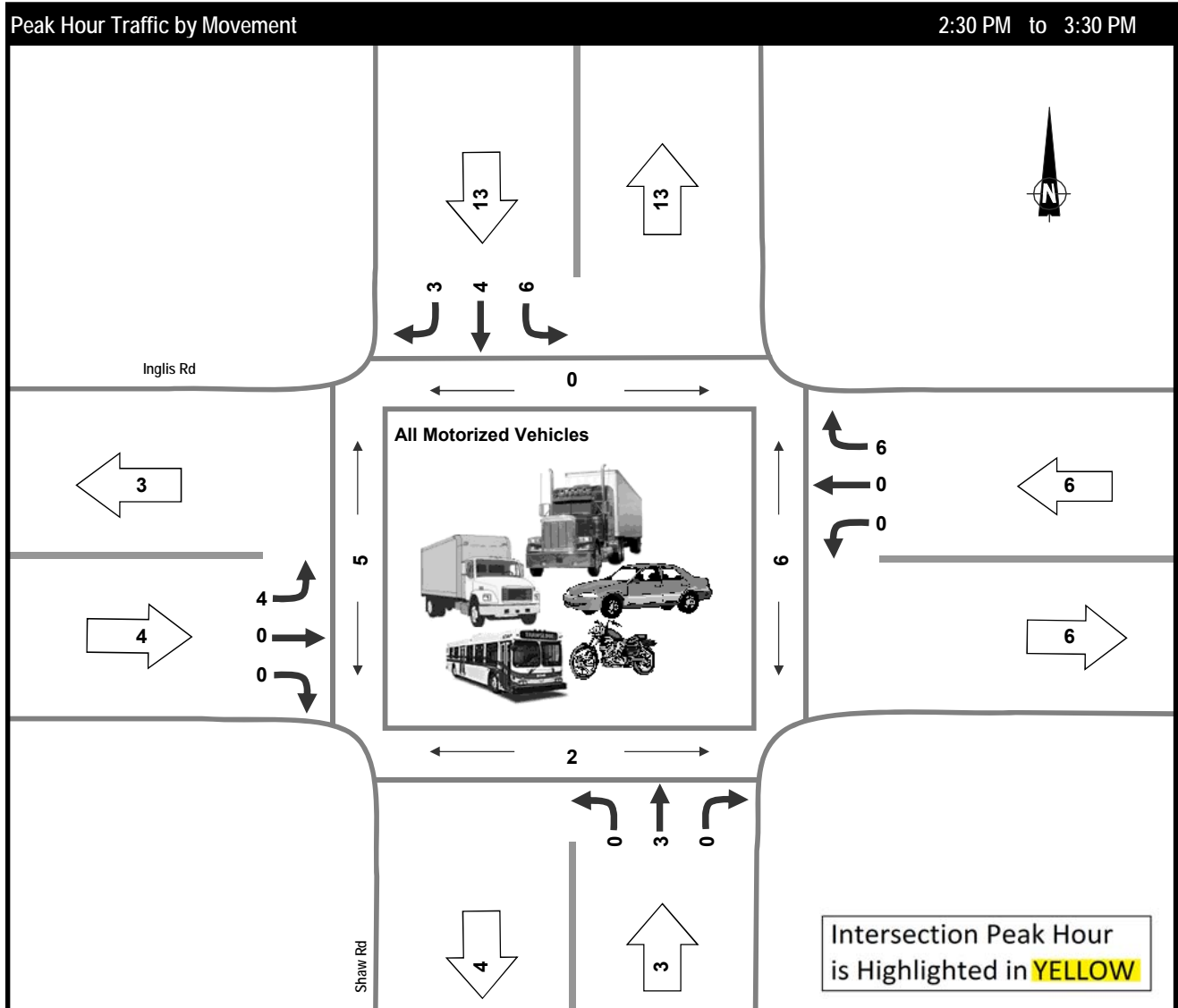
Vehicle Classification Summary

Project: #5602: 464 Eaglecrest Drive - Traffic Engineering Services
Municipality: Gibsons, BC
Weather: Clear, Sunny

Time Period	Entering Intersection	Vehicle Classification				Total
		Passenger Cars	Heavy Vehicles (3 or more axles)			
Morning	Volume					
	%					
Midday	Volume					
	%					
Afternoon (14:00 - 17:30)	Volume	68	0			68
	%	100.0%	0.0%			100.0%
Total (3.5 Hours)	Volume	68	0			68
	%	100.0%	0.0%			100.0%

Project: #5602: 464 Eaglecrest Drive - Traffic Engineering Services
Municipality: Gibsons, BC
Weather: Clear, Sunny
Vehicle Class: All Motorized Vehicles

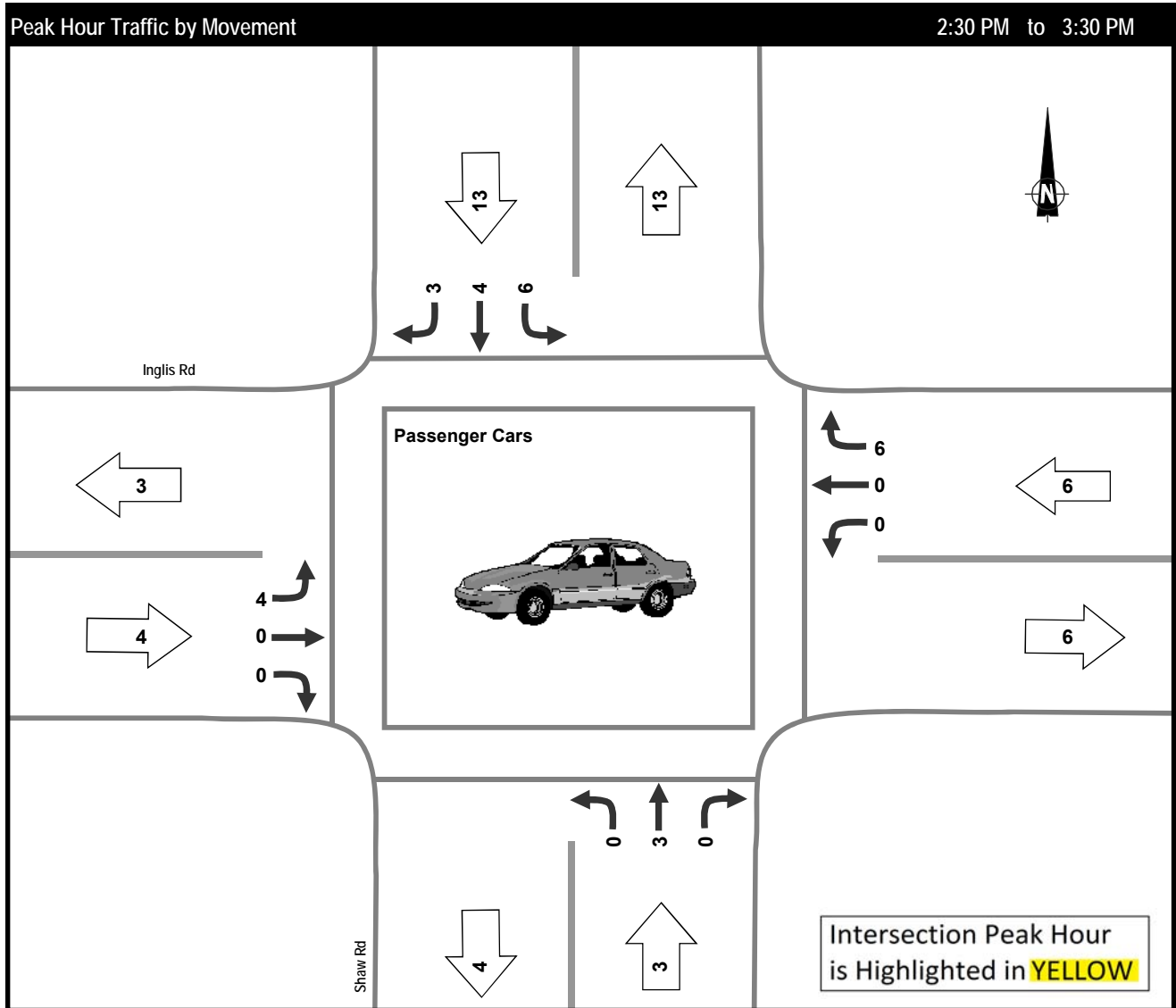
Afternoon Peak Period



Time	NORTH Approach			SOUTH Approach			WEST Approach			EAST Approach			PEDESTRIANS				Total Volumes
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	N	S	W	E	
Peak Hour	6	4	3	0	3	0	4	0	0	0	0	6	0	2	5	6	26
PHF	0.75	0.50	0.38	0.00	0.38	0.00	0.50	0.00	0.00	0.00	0.00	0.75	0.00	0.50	0.42	0.75	0.72
Peak 15 X 4	8	8	8	0	8	0	8	0	0	0	0	8	0	4	12	8	36
Average Hour	6	2	2	0	2	0	2	1	0	0	0	4	0	3	6	6	19
Survey Total	22	8	7	0	8	0	8	2	0	0	0	13	0	9	21	20	68
14:00	3	3	0	0	2	0	0	1	0	0	0	0	0	0	0	2	9
14:15	2	0	1	0	0	0	0	0	0	0	0	1	0	0	1	0	4
14:30	2	1	1	0	0	0	0	0	0	0	0	2	0	0	0	2	6
14:45	2	0	2	0	0	0	1	0	0	0	0	1	0	0	3	1	6
15:00	0	1	0	0	1	0	2	0	0	0	0	1	0	1	2	2	5
15:15	2	2	0	0	2	0	1	0	0	0	0	2	0	1	0	1	9
15:30	2	0	0	0	0	0	0	0	0	0	0	2	0	0	0	5	4
15:45	2	0	1	0	2	0	0	0	0	0	0	1	0	1	0	0	6
16:00	1	0	0	0	0	0	0	0	0	0	0	0	0	3	3	1	1
16:15	1	0	0	0	0	0	1	0	0	0	0	3	0	0	1	0	5
16:30	1	1	1	0	1	0	1	1	0	0	0	0	0	0	3	2	6
16:45	1	0	1	0	0	0	0	0	0	0	0	0	0	0	1	3	2
17:00	2	0	0	0	0	0	0	0	0	0	0	0	0	2	1	0	2
17:15	1	0	0	0	0	0	2	0	0	0	0	0	0	1	6	1	3

Project: #5602: 464 Eaglecrest Drive - Traffic Engineering Services
Municipality: Gibsons, BC
Weather: Clear, Sunny
Vehicle Class: Passenger Cars

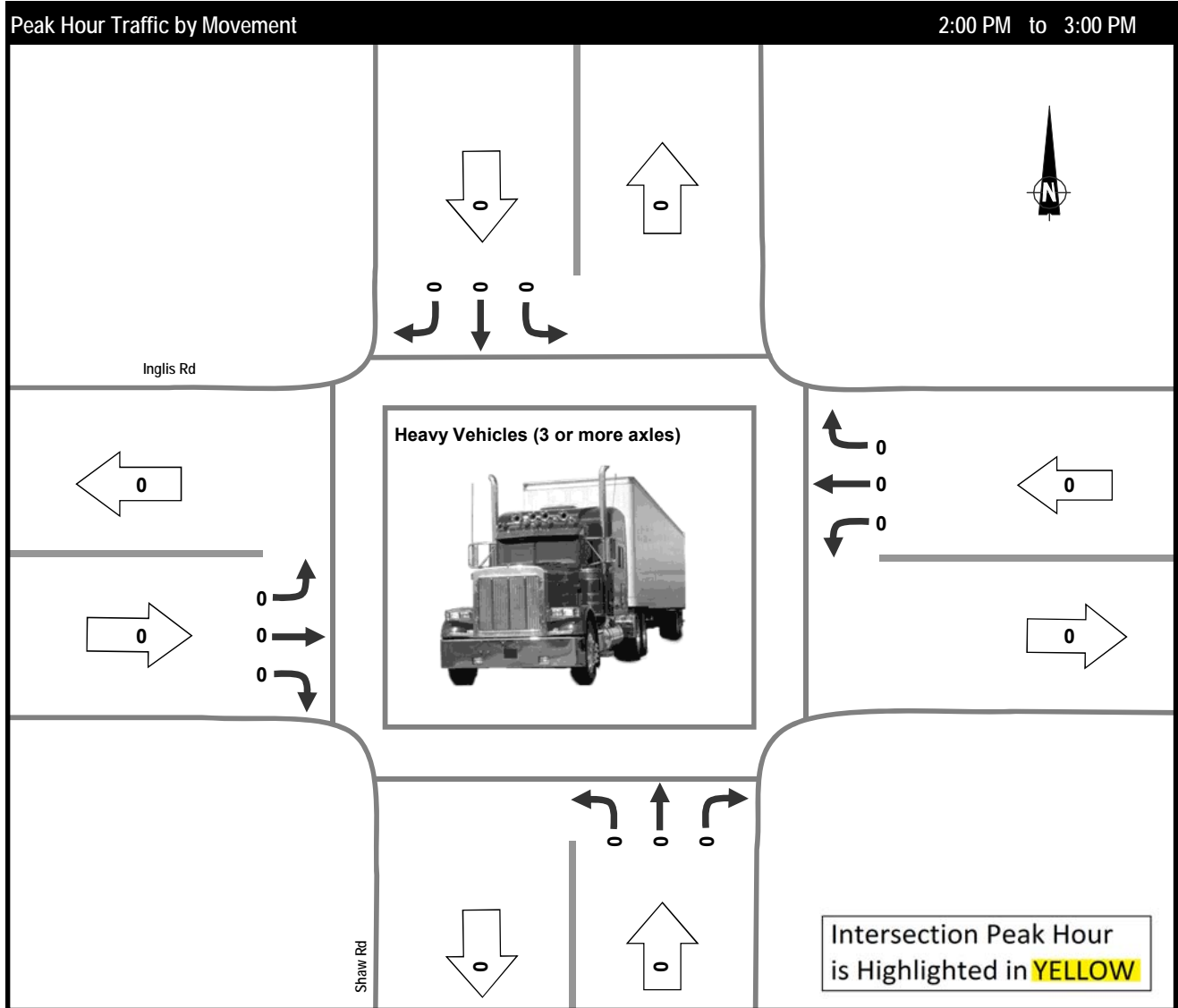
Afternoon Peak Period



Time	NORTH Approach			SOUTH Approach			WEST Approach			EAST Approach			PEDESTRIANS				Total Volumes
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	N	S	W	E	
Peak Hour	6	4	3	0	3	0	4	0	0	0	0	6					26
PHF	0.75	0.50	0.38	0.00	0.38	0.00	0.50	0.00	0.00	0.00	0.00	0.75					0.72
Peak 15 X 4	8	8	8	0	8	0	8	0	0	0	0	8					36
Average Hour	6	2	2	0	2	0	2	1	0	0	0	4					19
Survey Total	22	8	7	0	8	0	8	2	0	0	0	13					68
14:00	3	3	0	0	2	0	0	1	0	0	0	0					9
14:15	2	0	1	0	0	0	0	0	0	0	0	1					4
14:30	2	1	1	0	0	0	0	0	0	0	0	2					6
14:45	2	0	2	0	0	0	1	0	0	0	0	1					6
15:00	0	1	0	0	1	0	2	0	0	0	0	1					5
15:15	2	2	0	0	2	0	1	0	0	0	0	2					9
15:30	2	0	0	0	0	0	0	0	0	0	0	2					4
15:45	2	0	1	0	2	0	0	0	0	0	0	1					6
16:00	1	0	0	0	0	0	0	0	0	0	0	0					1
16:15	1	0	0	0	0	0	1	0	0	0	0	3					5
16:30	1	1	1	0	1	0	1	1	0	0	0	0					6
16:45	1	0	1	0	0	0	0	0	0	0	0	0					2
17:00	2	0	0	0	0	0	0	0	0	0	0	0					2
17:15	1	0	0	0	0	0	2	0	0	0	0	0					3

Project: #5602: 464 Eaglecrest Drive - Traffic Engineering Services
Municipality: Gibsons, BC
Weather: Clear, Sunny
Vehicle Class: Heavy Vehicles (3 or more axles)

Afternoon Peak Period



Time	NORTH Approach			SOUTH Approach			WEST Approach			EAST Approach			PEDESTRIANS				Total Volumes
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	N	S	W	E	
Peak Hour	0	0	0	0	0	0	0	0	0	0	0	0					0
PHF	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00					0.00
Peak 15 X 4	0	0	0	0	0	0	0	0	0	0	0	0					0
Average Hour	0	0	0	0	0	0	0	0	0	0	0	0					0
Survey Total	0	0	0	0	0	0	0	0	0	0	0	0					0
14:00	0	0	0	0	0	0	0	0	0	0	0	0					0
14:15	0	0	0	0	0	0	0	0	0	0	0	0					0
14:30	0	0	0	0	0	0	0	0	0	0	0	0					0
14:45	0	0	0	0	0	0	0	0	0	0	0	0					0
15:00	0	0	0	0	0	0	0	0	0	0	0	0					0
15:15	0	0	0	0	0	0	0	0	0	0	0	0					0
15:30	0	0	0	0	0	0	0	0	0	0	0	0					0
15:45	0	0	0	0	0	0	0	0	0	0	0	0					0
16:00	0	0	0	0	0	0	0	0	0	0	0	0					0
16:15	0	0	0	0	0	0	0	0	0	0	0	0					0
16:30	0	0	0	0	0	0	0	0	0	0	0	0					0
16:45	0	0	0	0	0	0	0	0	0	0	0	0					0
17:00	0	0	0	0	0	0	0	0	0	0	0	0					0
17:15	0	0	0	0	0	0	0	0	0	0	0	0					0

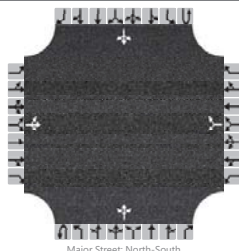
APPENDIX C

Intersection Capacity Analysis Worksheets

HCS7 Two-Way Stop-Control Report

General Information		Site Information	
Analyst	RC	Intersection	Shaw Rd & Inglis Rd
Agency/Co.	CTS	Jurisdiction	Gibsons, BC
Date Performed	Base	East/West Street	Inglis Road
Analysis Year	2017	North/South Street	Shaw Road
Time Analyzed	Weekday PM Peak Hour	Peak Hour Factor	0.82
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description		5602 - 464 Eaglecrest Dr Traffic Engineer Service	

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Movement																
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	1	0		0	1	0		0	1	0		0	1	0
Configuration			LTR				LR				LTR				LTR	
Volume, V (veh/h)		3	0	0		0		6		0	1	0		7	2	4
Percent Heavy Vehicles (%)		2	2	2		2		2						2		
Proportion Time Blocked																
Percent Grade (%)		0				0										
Right Turn Channelized		No				No				No						
Median Type/Storage	Undivided															

Critical and Follow-up Headways

Base Critical Headway (sec)																
Critical Headway (sec)																
Base Follow-Up Headway (sec)																
Follow-Up Headway (sec)																

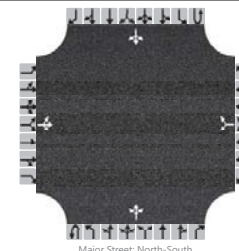
Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)			4			7			0					9			
Capacity, c (veh/h)			955			1069			1609					1620			
v/c Ratio			0.00			0.01			0.00					0.01			
95% Queue Length, Q ₉₅ (veh)			0.0			0.0			0.0					0.0			
Control Delay (s/veh)			8.8			8.4			7.2					7.2			
Level of Service, LOS			A			A			A					A			
Approach Delay (s/veh)		8.8				8.4				0.0				4.1			
Approach LOS		A				A											

HCS7 Two-Way Stop-Control Report

General Information		Site Information	
Analyst	RC	Intersection	Shaw Rd & Inglis Rd
Agency/Co.	CTS	Jurisdiction	Gibsons, BC
Date Performed	Base	East/West Street	Inglis Road
Analysis Year	2020	North/South Street	Shaw Road
Time Analyzed	Weekday PM Peak Hour	Peak Hour Factor	0.82
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description		5602 - 464 Eaglecrest Dr Traffic Engineer Service	

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Movement																
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	1	0		0	1	0		0	1	0		0	1	0
Configuration			LTR				LR				LTR				LTR	
Volume, V (veh/h)		4	0	0		0		6		0	1	0		7	2	5
Percent Heavy Vehicles (%)		2	2	2		2		2						2		
Proportion Time Blocked																
Percent Grade (%)		0				0										
Right Turn Channelized		No				No				No						
Median Type/Storage	Undivided															

Critical and Follow-up Headways

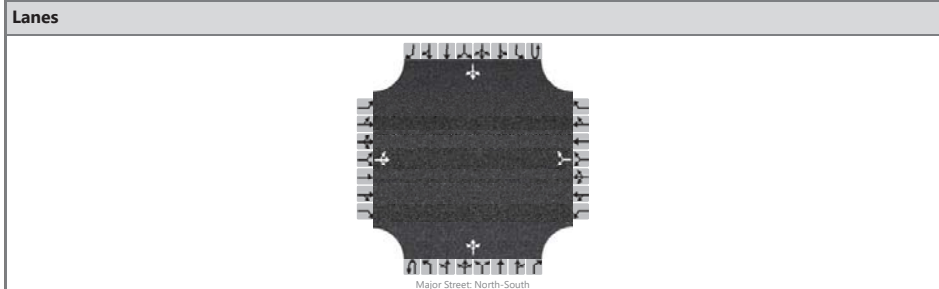
Base Critical Headway (sec)																
Critical Headway (sec)																
Base Follow-Up Headway (sec)																
Follow-Up Headway (sec)																

Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)			5			7			0					9			
Capacity, c (veh/h)			955			1069			1608					1620			
v/c Ratio			0.01			0.01			0.00					0.01			
95% Queue Length, Q ₉₅ (veh)			0.0			0.0			0.0					0.0			
Control Delay (s/veh)			8.8			8.4			7.2					7.2			
Level of Service, LOS			A			A			A					A			
Approach Delay (s/veh)		8.8				8.4				0.0				3.8			
Approach LOS		A				A											

HCS7 Two-Way Stop-Control Report

General Information		Site Information	
Analyst	RC	Intersection	Shaw Rd & Inglis Rd
Agency/Co.	CTS	Jurisdiction	Gibsons, BC
Date Performed	Base + Site	East/West Street	Inglis Road
Analysis Year	2020	North/South Street	Shaw Road
Time Analyzed	Weekday PM Peak Hour	Peak Hour Factor	0.82
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description	5602 - 464 Eaglecrest Dr Traffic Engineer Service		



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound				
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R	
Movement																	
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6	
Number of Lanes		0	1	0		0	1	0		0	1	0		0	1	0	
Configuration			LTR				LR				LTR				LTR		
Volume, V (veh/h)		4	0	0		0		13		0	1	0		18	2	5	
Percent Heavy Vehicles (%)		2	2	2		2		2						2			
Proportion Time Blocked																	
Percent Grade (%)		0				0											
Right Turn Channelized		No				No				No							
Median Type/Storage		Undivided															

Critical and Follow-up Headways

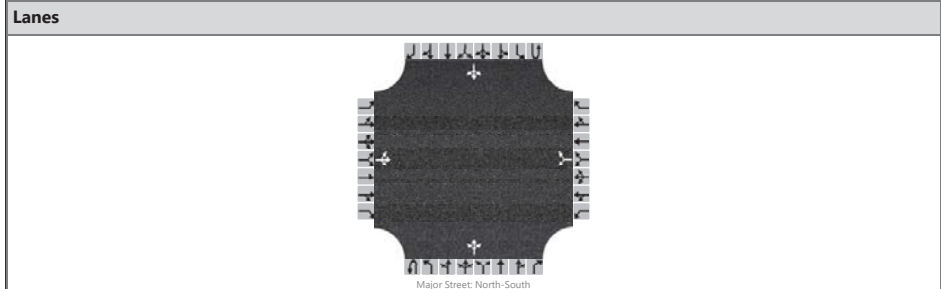
Base Critical Headway (sec)																	
Critical Headway (sec)																	
Base Follow-Up Headway (sec)																	
Follow-Up Headway (sec)																	

Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)			5			16			0					22			
Capacity, c (veh/h)			899			1069			1608					1620			
v/c Ratio			0.01			0.01			0.00					0.01			
95% Queue Length, Q ₉₅ (veh)			0.0			0.0			0.0					0.0			
Control Delay (s/veh)			9.0			8.4			7.2					7.3			
Level of Service, LOS			A			A			A					A			
Approach Delay (s/veh)		9.0				8.4				0.0				5.3			
Approach LOS		A				A											

HCS7 Two-Way Stop-Control Report

General Information		Site Information	
Analyst	RC	Intersection	Shaw Rd & Inglis Rd
Agency/Co.	CTS	Jurisdiction	Gibsons, BC
Date Performed	Base + Site	East/West Street	Inglis Road
Analysis Year	2025	North/South Street	Shaw Road
Time Analyzed	Weekday PM Peak Hour	Peak Hour Factor	0.82
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description	5602 - 464 Eaglecrest Dr Traffic Engineer Service		



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound				
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R	
Movement																	
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6	
Number of Lanes		0	1	0		0	1	0		0	1	0		0	1	0	
Configuration			LTR				LR				LTR				LTR		
Volume, V (veh/h)		4	0	0		0		14		0	1	0		19	3	5	
Percent Heavy Vehicles (%)		2	2	2		2		2		2				2			
Proportion Time Blocked																	
Percent Grade (%)		0				0											
Right Turn Channelized		No				No				No							
Median Type/Storage		Undivided															

Critical and Follow-up Headways

Base Critical Headway (sec)																	
Critical Headway (sec)																	
Base Follow-Up Headway (sec)																	
Follow-Up Headway (sec)																	

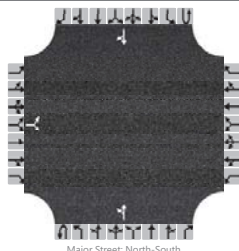
Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)			5			17			0					23			
Capacity, c (veh/h)			891			1069			1605					1620			
v/c Ratio			0.01			0.02			0.00					0.01			
95% Queue Length, Q ₉₅ (veh)			0.0			0.0			0.0					0.0			
Control Delay (s/veh)			9.1			8.4			7.2					7.3			
Level of Service, LOS			A			A			A					A			
Approach Delay (s/veh)		9.1				8.4				0.0				5.1			
Approach LOS		A				A											

HCS7 Two-Way Stop-Control Report

General Information		Site Information	
Analyst	RC	Intersection	Eaglecrest Dr & Inglis Rd
Agency/Co.	CTS	Jurisdiction	Gibsons, BC
Date Performed	Base	East/West Street	Inglis Road
Analysis Year	2017	North/South Street	Eaglecrest Drive
Time Analyzed	Weekday PM Peak Hour	Peak Hour Factor	0.82
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description	5602 - 464 Eaglecrest Dr Traffic Engineer Service		

Lanes



Major Street: North-South

Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound				
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R	
Movement																	
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6	
Number of Lanes		0	1	0		0	0	0		0	1	0		0	1	0	
Configuration			LR								LT					TR	
Volume, V (veh/h)		7		0						0	0				1	2	
Percent Heavy Vehicles (%)		2		2						2							
Proportion Time Blocked																	
Percent Grade (%)		0															
Right Turn Channelized		No					No					No					
Median Type/Storage		Undivided															

Critical and Follow-up Headways

Base Critical Headway (sec)																	
Critical Headway (sec)																	
Base Follow-Up Headway (sec)																	
Follow-Up Headway (sec)																	

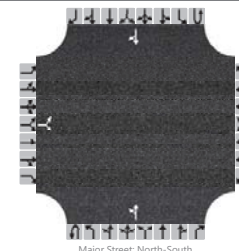
Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)			9								0						
Capacity, c (veh/h)			1020								1618						
v/c Ratio			0.01								0.00						
95% Queue Length, Q ₉₅ (veh)			0.0								0.0						
Control Delay (s/veh)			8.6								7.2						
Level of Service, LOS			A								A						
Approach Delay (s/veh)		8.6															
Approach LOS		A															

HCS7 Two-Way Stop-Control Report

General Information		Site Information	
Analyst	RC	Intersection	Eaglecrest Dr & Inglis Rd
Agency/Co.	CTS	Jurisdiction	Gibsons, BC
Date Performed	Base	East/West Street	Inglis Road
Analysis Year	2020	North/South Street	Eaglecrest Drive
Time Analyzed	Weekday PM Peak Hour	Peak Hour Factor	0.82
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description	5602 - 464 Eaglecrest Dr Traffic Engineer Service		

Lanes



Major Street: North-South

Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound				
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R	
Movement																	
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6	
Number of Lanes		0	1	0		0	0	0		0	1	0		0	1	0	
Configuration			LR								LT					TR	
Volume, V (veh/h)		7		0						0	0				1	2	
Percent Heavy Vehicles (%)		2		2						2							
Proportion Time Blocked																	
Percent Grade (%)		0															
Right Turn Channelized		No					No					No					
Median Type/Storage		Undivided															

Critical and Follow-up Headways

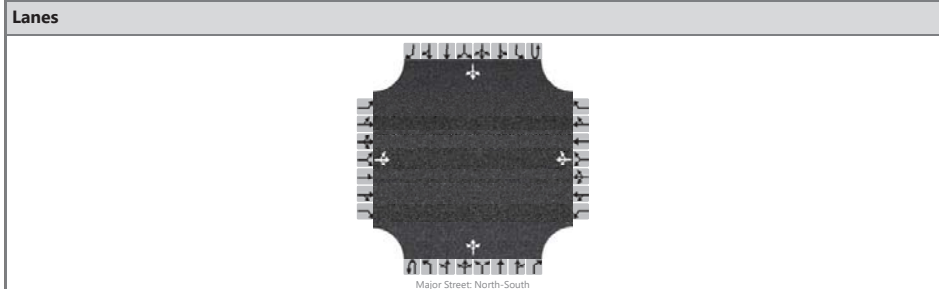
Base Critical Headway (sec)																	
Critical Headway (sec)																	
Base Follow-Up Headway (sec)																	
Follow-Up Headway (sec)																	

Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)			9								0						
Capacity, c (veh/h)			1020								1618						
v/c Ratio			0.01								0.00						
95% Queue Length, Q ₉₅ (veh)			0.0								0.0						
Control Delay (s/veh)			8.6								7.2						
Level of Service, LOS			A								A						
Approach Delay (s/veh)		8.6															
Approach LOS		A															

HCS7 Two-Way Stop-Control Report

General Information		Site Information	
Analyst	RC	Intersection	Eaglecrest Dr & Inglis Rd
Agency/Co.	CTS	Jurisdiction	Gibsons, BC
Date Performed	Base + Site	East/West Street	Inglis Road
Analysis Year	2020	North/South Street	Eaglecrest Drive
Time Analyzed	Weekday PM Peak Hour	Peak Hour Factor	0.82
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description		5602 - 464 Eaglecrest Dr Traffic Engineer Service	



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Movement																
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	1	0		0	1	0		0	1	0		0	1	0
Configuration			LTR				LTR				LTR				LTR	
Volume, V (veh/h)		7	11	0		0	7	6		0	0	0		12	1	2
Percent Heavy Vehicles (%)		2	2	2		2	2	2		2				2		
Proportion Time Blocked																
Percent Grade (%)		0				0										
Right Turn Channelized		No				No				No				No		
Median Type/Storage	Undivided															

Critical and Follow-up Headways

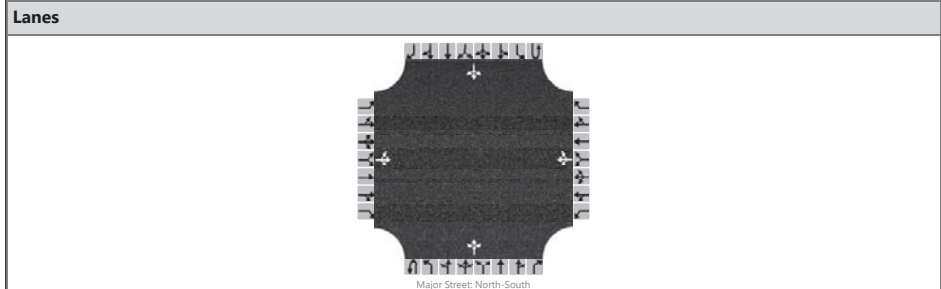
Base Critical Headway (sec)																
Critical Headway (sec)																
Base Follow-Up Headway (sec)																
Follow-Up Headway (sec)																

Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)		22			16				0				15			
Capacity, c (veh/h)		895			940				1618				1622			
v/c Ratio		0.02			0.02				0.00				0.01			
95% Queue Length, Q ₉₅ (veh)		0.1			0.1				0.0				0.0			
Control Delay (s/veh)		9.1			8.9				7.2				7.2			
Level of Service, LOS		A			A				A				A			
Approach Delay (s/veh)		9.1				8.9				6.0						
Approach LOS		A				A										

HCS7 Two-Way Stop-Control Report

General Information		Site Information	
Analyst	RC	Intersection	Eaglecrest Dr & Inglis Rd
Agency/Co.	CTS	Jurisdiction	Gibsons, BC
Date Performed	Base + Site	East/West Street	Inglis Road
Analysis Year	2025	North/South Street	Eaglecrest Drive
Time Analyzed	Weekday PM Peak Hour	Peak Hour Factor	0.82
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description		5602 - 464 Eaglecrest Dr Traffic Engineer Service	



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Movement																
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	1	0		0	1	0		0	1	0		0	1	0
Configuration			LTR				LTR				LTR				LTR	
Volume, V (veh/h)		8	11	0		0	7	6		0	0	0		12	1	3
Percent Heavy Vehicles (%)		2	2	2		2	2	2		2				2		
Proportion Time Blocked																
Percent Grade (%)		0				0										
Right Turn Channelized		No				No				No				No		
Median Type/Storage	Undivided															

Critical and Follow-up Headways

Base Critical Headway (sec)																
Critical Headway (sec)																
Base Follow-Up Headway (sec)																
Follow-Up Headway (sec)																

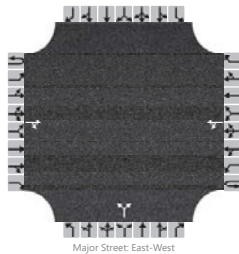
Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)		23			16				0				15			
Capacity, c (veh/h)		896			938				1615				1622			
v/c Ratio		0.03			0.02				0.00				0.01			
95% Queue Length, Q ₉₅ (veh)		0.1			0.1				0.0				0.0			
Control Delay (s/veh)		9.1			8.9				7.2				7.2			
Level of Service, LOS		A			A				A				A			
Approach Delay (s/veh)		9.1				8.9				5.4						
Approach LOS		A				A										

HCS7 Two-Way Stop-Control Report

General Information		Site Information	
Analyst	RC	Intersection	Eaglecrest Dr & O'Shea Rd
Agency/Co.	CTS	Jurisdiction	Gibsons, BC
Date Performed	Base	East/West Street	O'Shea Road
Analysis Year	2017	North/South Street	Eaglecrest Drive
Time Analyzed	Weekday PM Peak Hour	Peak Hour Factor	0.82
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description: 5602 - 464 Eaglecrest Dr Traffic Engineer Service			

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Movement	1U	1	2	3	4U	4	5	6	7	8	9		10	11	12	
Priority																
Number of Lanes	0	0	1	0	0	0	1	0	0	1	0		0	0	0	
Configuration				TR			LT				LR					
Volume, V (veh/h)			59	9		12	63			11	17					
Percent Heavy Vehicles (%)						2				2						
Proportion Time Blocked																
Percent Grade (%)										2						
Right Turn Channelized			No			No				No				No		
Median Type/Storage				Undivided												

Critical and Follow-up Headways

Base Critical Headway (sec)																
Critical Headway (sec)																
Base Follow-Up Headway (sec)																
Follow-Up Headway (sec)																

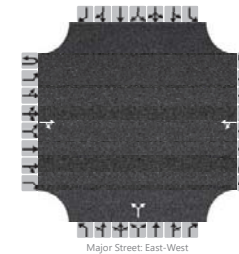
Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)					15				34							
Capacity, c (veh/h)					1489				865							
v/c Ratio					0.01				0.04							
95% Queue Length, Q ₉₅ (veh)					0.0				0.1							
Control Delay (s/veh)					7.4				9.3							
Level of Service, LOS					A				A							
Approach Delay (s/veh)					1.3				9.3							
Approach LOS									A							

HCS7 Two-Way Stop-Control Report

General Information		Site Information	
Analyst	RC	Intersection	Eaglecrest Dr & O'Shea Rd
Agency/Co.	CTS	Jurisdiction	Gibsons, BC
Date Performed	Base	East/West Street	O'Shea Road
Analysis Year	2020	North/South Street	Eaglecrest Drive
Time Analyzed	Weekday PM Peak Hour	Peak Hour Factor	0.82
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description: 5602 - 464 Eaglecrest Dr Traffic Engineer Service			

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Movement	1U	1	2	3	4U	4	5	6	7	8	9		10	11	12	
Priority																
Number of Lanes	0	0	1	0	0	0	1	0	0	1	0		0	0	0	
Configuration				TR			LT				LR					
Volume, V (veh/h)			64	10		13	67			12	18					
Percent Heavy Vehicles (%)						2				2	2					
Proportion Time Blocked																
Percent Grade (%)										2						
Right Turn Channelized			No			No				No				No		
Median Type/Storage				Undivided												

Critical and Follow-up Headways

Base Critical Headway (sec)																
Critical Headway (sec)																
Base Follow-Up Headway (sec)																
Follow-Up Headway (sec)																

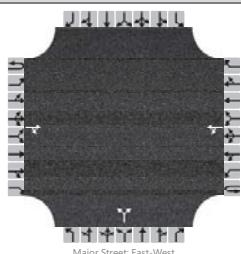
Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)					16				37							
Capacity, c (veh/h)					1480				848							
v/c Ratio					0.01				0.04							
95% Queue Length, Q ₉₅ (veh)					0.0				0.1							
Control Delay (s/veh)					7.5				9.4							
Level of Service, LOS					A				A							
Approach Delay (s/veh)					1.3				9.4							
Approach LOS									A							

HCS7 Two-Way Stop-Control Report

General Information		Site Information	
Analyst	RC	Intersection	Eaglecrest Dr & O'Shea Rd
Agency/Co.	CTS	Jurisdiction	Gibsons, BC
Date Performed	Base + Site	East/West Street	O'Shea Road
Analysis Year	2020	North/South Street	Eaglecrest Drive
Time Analyzed	Weekday PM Peak Hour	Peak Hour Factor	0.82
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description		5602 - 464 Eaglecrest Dr Traffic Engineer Service	

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Movement	1U	1	2	3	4U	4	5	6	7	8	9		10	11	12	
Priority																
Number of Lanes	0	0	1	0	0	0	1	0	0	1	0		0	0	0	
Configuration				TR			LT				LR					
Volume, V (veh/h)			64	11	24	67			12		24					
Percent Heavy Vehicles (%)						2				2						
Proportion Time Blocked																
Percent Grade (%)									2							
Right Turn Channelized		No				No				No				No		
Median Type/Storage						Undivided										

Critical and Follow-up Headways

Base Critical Headway (sec)																
Critical Headway (sec)																
Base Follow-Up Headway (sec)																
Follow-Up Headway (sec)																

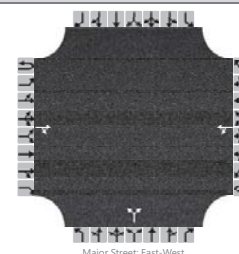
Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)					29				44							
Capacity, c (veh/h)					1479				845							
v/c Ratio					0.02				0.05							
95% Queue Length, Q ₉₅ (veh)					0.1				0.2							
Control Delay (s/veh)					7.5				9.5							
Level of Service, LOS					A				A							
Approach Delay (s/veh)					2.1				9.5							
Approach LOS									A							

HCS7 Two-Way Stop-Control Report

General Information		Site Information	
Analyst	RC	Intersection	Eaglecrest Dr & O'Shea Rd
Agency/Co.	CTS	Jurisdiction	Gibsons, BC
Date Performed	Base + Site	East/West Street	O'Shea Road
Analysis Year	2025	North/South Street	Eaglecrest Drive
Time Analyzed	Weekday PM Peak Hour	Peak Hour Factor	0.82
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description		5602 - 464 Eaglecrest Dr Traffic Engineer Service	

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Movement	1U	1	2	3	4U	4	5	6	7	8	9		10	11	12	
Priority																
Number of Lanes	0	0	1	0	0	0	1	0	0	1	0		0	0	0	
Configuration				TR			LT				LR					
Volume, V (veh/h)			71	12	26	75			13		26					
Percent Heavy Vehicles (%)						2				2						
Proportion Time Blocked																
Percent Grade (%)									2							
Right Turn Channelized		No				No				No				No		
Median Type/Storage						Undivided										

Critical and Follow-up Headways

Base Critical Headway (sec)																
Critical Headway (sec)																
Base Follow-Up Headway (sec)																
Follow-Up Headway (sec)																

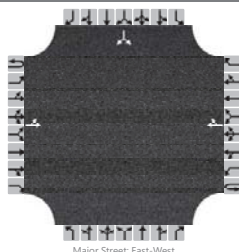
Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)					32				48							
Capacity, c (veh/h)					1465				827							
v/c Ratio					0.02				0.06							
95% Queue Length, Q ₉₅ (veh)					0.1				0.2							
Control Delay (s/veh)					7.5				9.6							
Level of Service, LOS					A				A							
Approach Delay (s/veh)					2.1				9.6							
Approach LOS									A							

HCS7 Two-Way Stop-Control Report

General Information		Site Information	
Analyst	RC	Intersection	Abbs Rd & Winn Rd
Agency/Co.	CTS	Jurisdiction	Gibsons, BC
Date Performed	Base	East/West Street	Winn Road
Analysis Year	2017	North/South Street	Abbs Road
Time Analyzed	Weekday PM Peak Hour	Peak Hour Factor	0.82
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	5602 - 464 Eaglecrest Dr Traffic Engineer Service		

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Movement	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Priority																
Number of Lanes	0	0	1	0	0	0	1	0		0	0	0		0	1	0
Configuration	LT								TR				LR			
Volume, V (veh/h)	8	0				0	25							16	8	
Percent Heavy Vehicles (%)	2													2		2
Proportion Time Blocked																
Percent Grade (%)																-8
Right Turn Channelized	No				No				No				No			
Median Type/Storage	Undivided															

Critical and Follow-up Headways

Base Critical Headway (sec)																
Critical Headway (sec)																
Base Follow-Up Headway (sec)																
Follow-Up Headway (sec)																

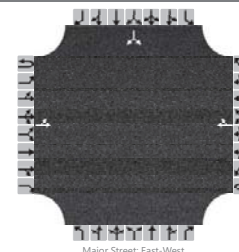
Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)	10															30
Capacity, c (veh/h)	1582															997
v/c Ratio	0.01															0.03
95% Queue Length, Q ₉₅ (veh)	0.0															0.1
Control Delay (s/veh)	7.3															8.7
Level of Service, LOS	A															A
Approach Delay (s/veh)	7.3												8.7			
Approach LOS													A			

HCS7 Two-Way Stop-Control Report

General Information		Site Information	
Analyst	RC	Intersection	Abbs Rd & Winn Rd
Agency/Co.	CTS	Jurisdiction	Gibsons, BC
Date Performed	Base	East/West Street	Winn Road
Analysis Year	2020	North/South Street	Abbs Road
Time Analyzed	Weekday PM Peak Hour	Peak Hour Factor	0.82
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	5602 - 464 Eaglecrest Dr Traffic Engineer Service		

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Movement	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Priority																
Number of Lanes	0	0	1	0	0	0	1	0		0	0	0		0	1	0
Configuration	LT								TR				LR			
Volume, V (veh/h)	8	0				0	26							17	8	
Percent Heavy Vehicles (%)	2													2		2
Proportion Time Blocked																
Percent Grade (%)																-8
Right Turn Channelized	No				No				No				No			
Median Type/Storage	Undivided															

Critical and Follow-up Headways

Base Critical Headway (sec)																
Critical Headway (sec)																
Base Follow-Up Headway (sec)																
Follow-Up Headway (sec)																

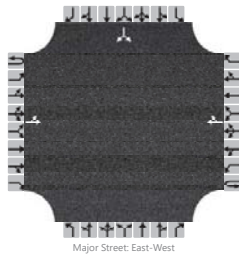
Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)	10															31
Capacity, c (veh/h)	1579															995
v/c Ratio	0.01															0.03
95% Queue Length, Q ₉₅ (veh)	0.0															0.1
Control Delay (s/veh)	7.3															8.7
Level of Service, LOS	A															A
Approach Delay (s/veh)	7.3												8.7			
Approach LOS													A			

HCS7 Two-Way Stop-Control Report

General Information		Site Information	
Analyst	RC	Intersection	Abbs Rd & Winn Rd
Agency/Co.	CTS	Jurisdiction	Gibsons, BC
Date Performed	Base + Site	East/West Street	Winn Road
Analysis Year	2020	North/South Street	Abbs Road
Time Analyzed	Weekday PM Peak Hour	Peak Hour Factor	0.82
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description 5602 - 464 Eaglecrest Dr Traffic Engineer Service			

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Movement	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Priority																
Number of Lanes	0	0	1	0	0	0	1	0		0	0	0		0	1	0
Configuration	LT								TR				LR			
Volume, V (veh/h)	9	4					8	26						17		9
Percent Heavy Vehicles (%)	2													2		2
Proportion Time Blocked																
Percent Grade (%)																-8
Right Turn Channelized	No				No				No				No			
Median Type/Storage	Undivided															

Critical and Follow-up Headways

Base Critical Headway (sec)																
Critical Headway (sec)																
Base Follow-Up Headway (sec)																
Follow-Up Headway (sec)																

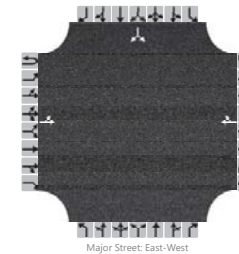
Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)	11															32
Capacity, c (veh/h)	1566															982
v/c Ratio	0.01															0.03
95% Queue Length, Q ₉₅ (veh)	0.0															0.1
Control Delay (s/veh)	7.3															8.8
Level of Service, LOS	A															A
Approach Delay (s/veh)	5.0												8.8			
Approach LOS													A			

HCS7 Two-Way Stop-Control Report

General Information		Site Information	
Analyst	RC	Intersection	Abbs Rd & Winn Rd
Agency/Co.	CTS	Jurisdiction	Gibsons, BC
Date Performed	Base + Site	East/West Street	Winn Road
Analysis Year	2025	North/South Street	Abbs Road
Time Analyzed	Weekday PM Peak Hour	Peak Hour Factor	0.82
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description 5602 - 464 Eaglecrest Dr Traffic Engineer Service			

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Movement	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Priority																
Number of Lanes	0	0	1	0	0	0	1	0		0	0	0		0	1	0
Configuration	LT								TR				LR			
Volume, V (veh/h)	10	4					8	30						19		10
Percent Heavy Vehicles (%)	2													2		2
Proportion Time Blocked																
Percent Grade (%)																-8
Right Turn Channelized	No				No				No				No			
Median Type/Storage	Undivided															

Critical and Follow-up Headways

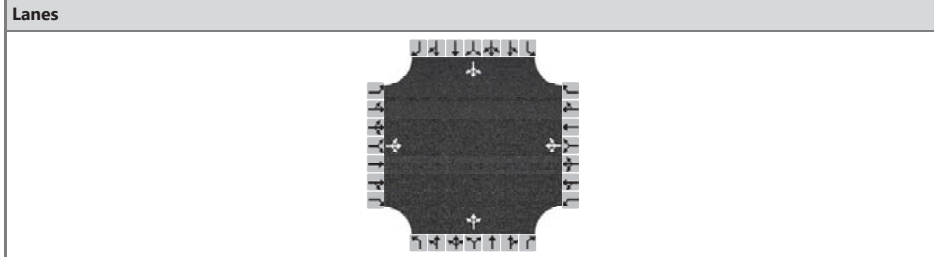
Base Critical Headway (sec)																
Critical Headway (sec)																
Base Follow-Up Headway (sec)																
Follow-Up Headway (sec)																

Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)	12															35
Capacity, c (veh/h)	1559															977
v/c Ratio	0.01															0.04
95% Queue Length, Q ₉₅ (veh)	0.0															0.1
Control Delay (s/veh)	7.3															8.8
Level of Service, LOS	A															A
Approach Delay (s/veh)	5.2												8.8			
Approach LOS													A			

HCS7 All-Way Stop Control Report

General Information		Site Information	
Analyst	RC	Intersection	Gower Point Rd & Winn Rd
Agency/Co.	CTS	Jurisdiction	Gibsons, BC
Date Performed	Base	East/West Street	Winn Road
Analysis Year	2017	North/South Street	Gower Point Road
Analysis Time Period (hrs)	0.25	Peak Hour Factor	0.82
Time Analyzed	Weekday PM Peak Hour		
Project Description	5602 - 464 Eaglecrest Drive Traffic Eng. Service		



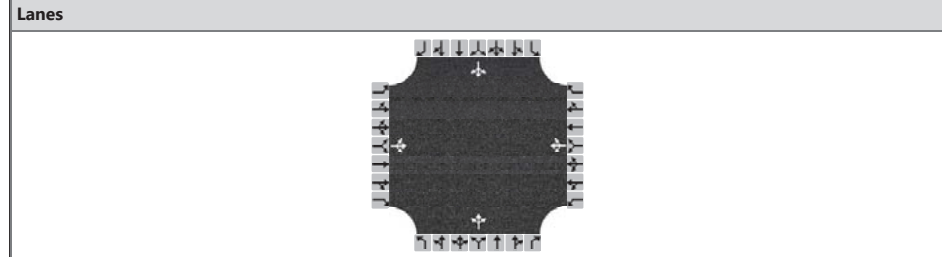
Approach	Eastbound			Westbound			Northbound			Southbound		
	L	T	R	L	T	R	L	T	R	L	T	R
Movement												
Volume	37	0	24	0	0	1	24	124	0	0	133	72
% Thrus in Shared Lane												
Lane	L1	L2	L3	L1	L2	L3	L1	L2	L3	L1	L2	L3
Configuration	LTR			LTR			LTR			LTR		
Flow Rate, v (veh/h)	74			1			180			250		
Percent Heavy Vehicles	2			2			2			2		

Departure Headway and Service Time												
Initial Departure Headway, hd (s)	3.20			3.20			3.20			3.20		
Initial Degree of Utilization, x	0.066			0.001			0.160			0.222		
Final Departure Headway, hd (s)	4.76			4.37			4.41			4.11		
Final Degree of Utilization, x	0.098			0.001			0.221			0.285		
Move-Up Time, m (s)	2.0			2.0			2.0			2.0		
Service Time, ts (s)	2.76			2.37			2.41			2.11		

Capacity, Delay and Level of Service												
Flow Rate, v (veh/h)	74			1			180			250		
Capacity	757			823			817			877		
95% Queue Length, Q ₉₅ (veh)	0.3			0.0			0.8			1.2		
Control Delay (s/veh)	8.3			7.4			8.7			8.7		
Level of Service, LOS	A			A			A			A		
Approach Delay (s/veh)	8.3			7.4			8.7			8.7		
Approach LOS	A			A			A			A		
Intersection Delay, s/veh LOS	8.6						A					

HCS7 All-Way Stop Control Report

General Information		Site Information	
Analyst	RC	Intersection	Gower Point Rd & Winn Rd
Agency/Co.	CTS	Jurisdiction	Gibsons, BC
Date Performed	Base	East/West Street	Winn Road
Analysis Year	2020	North/South Street	Gower Point Road
Analysis Time Period (hrs)	0.25	Peak Hour Factor	0.82
Time Analyzed	Weekday PM Peak Hour		
Project Description	5602 - 464 Eaglecrest Drive Traffic Eng. Service		



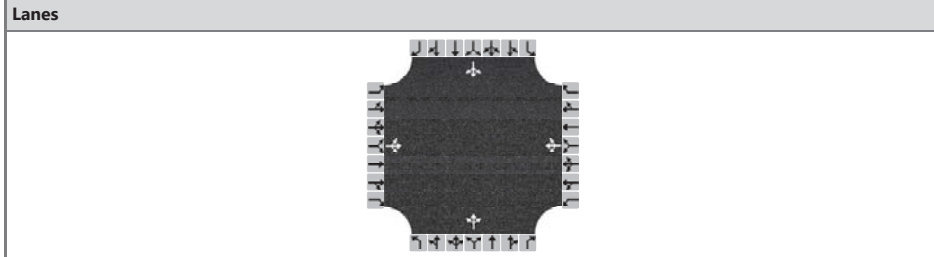
Approach	Eastbound			Westbound			Northbound			Southbound		
	L	T	R	L	T	R	L	T	R	L	T	R
Movement												
Volume	40	0	25	0	0	1	25	134	0	0	143	77
% Thrus in Shared Lane												
Lane	L1	L2	L3	L1	L2	L3	L1	L2	L3	L1	L2	L3
Configuration	LTR			LTR			LTR			LTR		
Flow Rate, v (veh/h)	79			1			194			268		
Percent Heavy Vehicles	2			2			2			2		

Departure Headway and Service Time												
Initial Departure Headway, hd (s)	3.20			3.20			3.20			3.20		
Initial Degree of Utilization, x	0.070			0.001			0.172			0.238		
Final Departure Headway, hd (s)	4.84			4.46			4.44			4.14		
Final Degree of Utilization, x	0.106			0.002			0.239			0.308		
Move-Up Time, m (s)	2.0			2.0			2.0			2.0		
Service Time, ts (s)	2.84			2.46			2.44			2.14		

Capacity, Delay and Level of Service												
Flow Rate, v (veh/h)	79			1			194			268		
Capacity	744			807			810			870		
95% Queue Length, Q ₉₅ (veh)	0.4			0.0			0.9			1.3		
Control Delay (s/veh)	8.4			7.5			8.8			9.0		
Level of Service, LOS	A			A			A			A		
Approach Delay (s/veh)	8.4			7.5			8.8			9.0		
Approach LOS	A			A			A			A		
Intersection Delay, s/veh LOS	8.8						A					

HCS7 All-Way Stop Control Report

General Information		Site Information	
Analyst	RC	Intersection	Gower Point Rd & Winn Rd
Agency/Co.	CTS	Jurisdiction	Gibsons, BC
Date Performed	Base + Sit	East/West Street	Winn Road
Analysis Year	2020	North/South Street	Gower Point Road
Analysis Time Period (hrs)	0.25	Peak Hour Factor	0.82
Time Analyzed	Weekday PM Peak Hour		
Project Description	5602 - 464 Eaglecrest Drive Traffic Eng. Service		



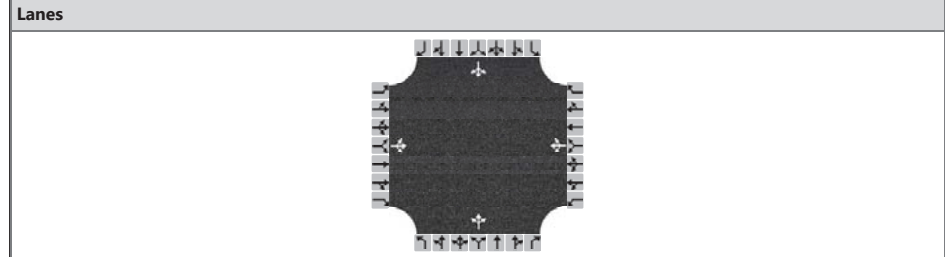
Vehicle Volume and Adjustments												
Approach	Eastbound			Westbound			Northbound			Southbound		
	L	T	R	L	T	R	L	T	R	L	T	R
Movement												
Volume	42	0	27	0	0	1	28	134	0	0	143	82
% Thrus in Shared Lane												
Lane	L1	L2	L3	L1	L2	L3	L1	L2	L3	L1	L2	L3
Configuration	LTR			LTR			LTR			LTR		
Flow Rate, v (veh/h)	84			1			198			274		
Percent Heavy Vehicles	2			2			2			2		

Departure Headway and Service Time												
Initial Departure Headway, hd (s)	3.20			3.20			3.20			3.20		
Initial Degree of Utilization, x	0.075			0.001			0.176			0.244		
Final Departure Headway, hd (s)	4.85			4.49			4.47			4.15		
Final Degree of Utilization, x	0.113			0.002			0.245			0.316		
Move-Up Time, m (s)	2.0			2.0			2.0			2.0		
Service Time, ts (s)	2.85			2.49			2.47			2.15		

Capacity, Delay and Level of Service												
Flow Rate, v (veh/h)	84			1			198			274		
Capacity	742			802			806			868		
95% Queue Length, Q ₉₅ (veh)	0.4			0.0			1.0			1.4		
Control Delay (s/veh)	8.5			7.5			8.9			9.1		
Level of Service, LOS	A			A			A			A		
Approach Delay (s/veh)	8.5			7.5			8.9			9.1		
Approach LOS	A			A			A			A		
Intersection Delay, s/veh LOS	8.9			A			A			A		

HCS7 All-Way Stop Control Report

General Information		Site Information	
Analyst	RC	Intersection	Gower Point Rd & Winn Rd
Agency/Co.	CTS	Jurisdiction	Gibsons, BC
Date Performed	Base+Site	East/West Street	Winn Road
Analysis Year	2025	North/South Street	Gower Point Road
Analysis Time Period (hrs)	0.25	Peak Hour Factor	0.82
Time Analyzed	Weekday PM Peak Hour		
Project Description	5602 - 464 Eaglecrest Drive Traffic Eng. Service		



Vehicle Volume and Adjustments												
Approach	Eastbound			Westbound			Northbound			Southbound		
	L	T	R	L	T	R	L	T	R	L	T	R
Movement												
Volume	46	0	30	0	0	1	31	149	0	0	160	91
% Thrus in Shared Lane												
Lane	L1	L2	L3	L1	L2	L3	L1	L2	L3	L1	L2	L3
Configuration	LTR			LTR			LTR			LTR		
Flow Rate, v (veh/h)	93			1			220			306		
Percent Heavy Vehicles	2			2			2			2		

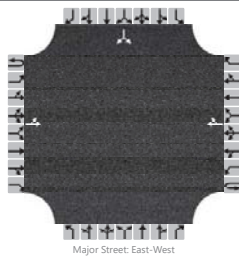
Departure Headway and Service Time												
Initial Departure Headway, hd (s)	3.20			3.20			3.20			3.20		
Initial Degree of Utilization, x	0.082			0.001			0.195			0.272		
Final Departure Headway, hd (s)	4.98			4.64			4.53			4.21		
Final Degree of Utilization, x	0.128			0.002			0.276			0.358		
Move-Up Time, m (s)	2.0			2.0			2.0			2.0		
Service Time, ts (s)	2.98			2.64			2.53			2.21		

Capacity, Delay and Level of Service												
Flow Rate, v (veh/h)	93			1			220			306		
Capacity	723			776			794			856		
95% Queue Length, Q ₉₅ (veh)	0.4			0.0			1.1			1.6		
Control Delay (s/veh)	8.7			7.6			9.3			9.5		
Level of Service, LOS	A			A			A			A		
Approach Delay (s/veh)	8.7			7.6			9.3			9.5		
Approach LOS	A			A			A			A		
Intersection Delay, s/veh LOS	9.3			A			A			A		

HCS7 Two-Way Stop-Control Report

General Information		Site Information	
Analyst	RC	Intersection	Site Access & Winn Rd
Agency/Co.	CTS	Jurisdiction	Gibsons, BC
Date Performed	Base+Site	East/West Street	Winn Road
Analysis Year	2020	North/South Street	Site Access
Time Analyzed	Weekday PM Peak Hour	Peak Hour Factor	0.82
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description		5602 - 464 Eaglecrest Dr Traffic Engineer Service	

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Movement	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Priority																
Number of Lanes	0	0	1	0	0	0	1	0		0	0	0		0	1	0
Configuration	LT								TR				LR			
Volume, V (veh/h)	0	8				8	9							5	0	0
Percent Heavy Vehicles (%)	2												2			
Proportion Time Blocked																
Percent Grade (%)													0			
Right Turn Channelized	No				No				No				No			
Median Type/Storage	Undivided															

Critical and Follow-up Headways

Base Critical Headway (sec)																
Critical Headway (sec)																
Base Follow-Up Headway (sec)																
Follow-Up Headway (sec)																

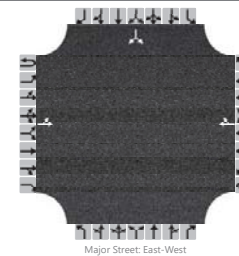
Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)	0														6	
Capacity, c (veh/h)	1594														965	
v/c Ratio	0.00														0.01	
95% Queue Length, Q ₉₅ (veh)	0.0														0.0	
Control Delay (s/veh)	7.3														8.8	
Level of Service, LOS	A														A	
Approach Delay (s/veh)	0.0												8.8			
Approach LOS													A			

HCS7 Two-Way Stop-Control Report

General Information		Site Information	
Analyst	RC	Intersection	Site Access & Winn Rd
Agency/Co.	CTS	Jurisdiction	Gibsons, BC
Date Performed	Base+Site	East/West Street	Winn Road
Analysis Year	2025	North/South Street	Site Access
Time Analyzed	Weekday PM Peak Hour	Peak Hour Factor	0.82
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description		5602 - 464 Eaglecrest Dr Traffic Engineer Service	

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Movement	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Priority																
Number of Lanes	0	0	1	0	0	0	1	0		0	0	0		0	1	0
Configuration	LT								TR				LR			
Volume, V (veh/h)	0	9				9	9							5	0	0
Percent Heavy Vehicles (%)	2												2			
Proportion Time Blocked																
Percent Grade (%)													0			
Right Turn Channelized	No				No				No				No			
Median Type/Storage	Undivided															

Critical and Follow-up Headways

Base Critical Headway (sec)																
Critical Headway (sec)																
Base Follow-Up Headway (sec)																
Follow-Up Headway (sec)																

Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)	0														6	
Capacity, c (veh/h)	1592														962	
v/c Ratio	0.00														0.01	
95% Queue Length, Q ₉₅ (veh)	0.0														0.0	
Control Delay (s/veh)	7.3														8.8	
Level of Service, LOS	A														A	
Approach Delay (s/veh)	0.0												8.8			
Approach LOS													A			

Date: 4 April 2018
Our File No: 5602-07

BY EMAIL

Mr. Rob Chetner
TCD Developments
1754 West 3rd Avenue
Vancouver, BC,
V6J 1K4

Dear Mr. Chetner

**Re: 464 Eaglecrest Drive Development - Follow up Traffic Engineering Comments
Sechelt, BC**

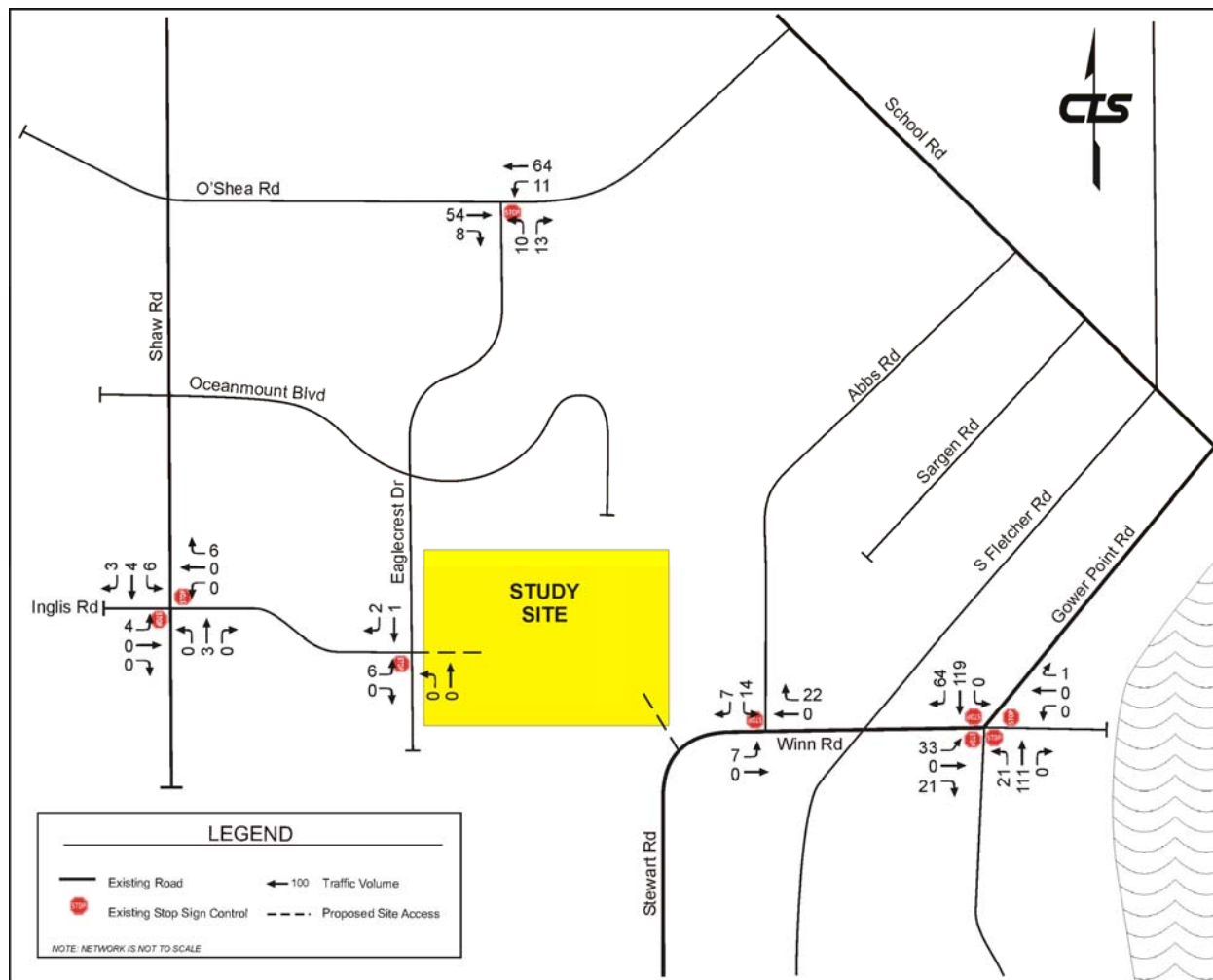
As requested, Creative Transportation Solutions Ltd. (CTS) has prepared this letter to provide some additional commentary to supplement the traffic impact study that was prepared and issued by CTS on 12 December 2017. The commentary contained in this letter is in regards to the volume of traffic that is currently in the study area versus the forecast volume of traffic after the proposed development is fully constructed and occupied.

For the traffic impact study, CTS conducted intersection traffic movement counts to document existing conditions in the study area on Friday, 13 October 2017. This is the Friday of the Thanksgiving holiday weekend and which is considered The traffic counts were undertaken from 14:00 to 17:30 in order to capture school peak and residential peak periods, as well as ferry traffic from Horseshoe Bay, when the adjacent road network is typically most congested. On the survey day, it was confirmed with School District 46 that schools were in session and that there were no significant ferry disruptions noted on the survey day which could have negatively impacted traffic volumes in the study area. The traffic count data was tabulated and reviewed by a professional traffic engineer to ensure data integrity and validity.

The tabulated traffic movement count data sheets were included in the traffic impact study report as Appendix B and which are also appended to this letter for ease of reference. The highest measured traffic volumes in the study area occurred between 2:15 and 3:30 pm for both the upper and lower neighbourhoods, which likely coincided with school pickup and departure traffic. **FIGURE 1** illustrates the measured Friday individual afternoon peak hour traffic volumes for both the upper and lower neighbourhoods in the study area from 13 October 2017.

It is important to note that for residential neighbourhoods similar to those surveyed in the traffic impact study, the weekday afternoon peak hour has been consistently shown to be the hour carrying the highest traffic volumes for the entire day. The remaining 23 hours each weekday including the morning peak hour normally carry much lower traffic volumes. This is why the BC Ministry of Transportation & Infrastructure requires that traffic impact studies on the Sunshine Coast always use the weekday afternoon peak hour as the design hour.

FIGURE 1
MEASURED TRAFFIC VOLUMES FOR FRIDAY, 13 OCTOBER 2017
INDIVIDUAL AFTERNOON PEAK HOURS



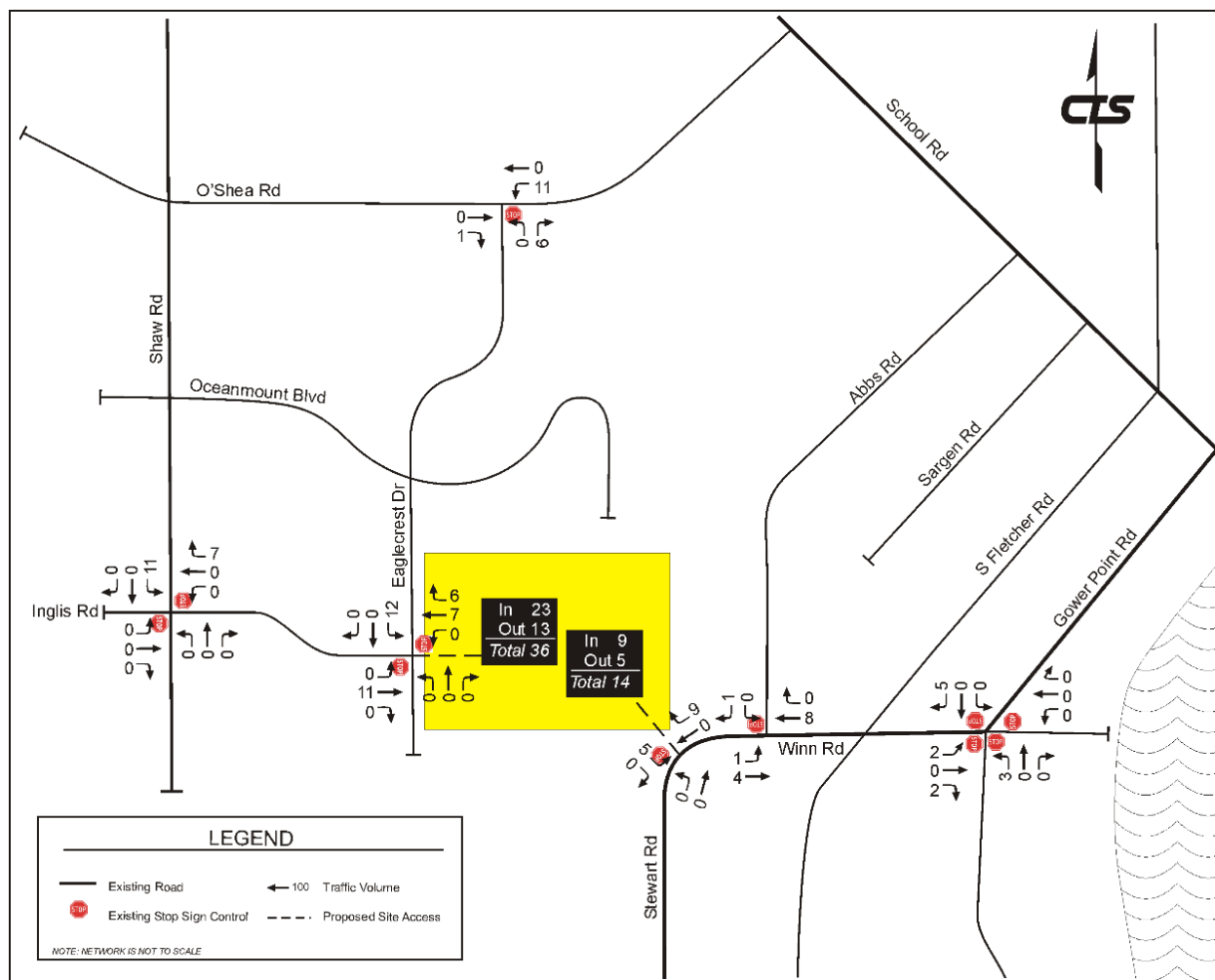
In reviewing the measured traffic volumes from the study area, the following observations can be made:

1. Inglis Road between Eaglecrest Drive and Shaw Road had measured two-way traffic volumes of 12 vehicles per hour at Shaw Road and 8 vehicles per hour at Eaglecrest Drive. Just east of Shaw Road, Inglis Road was carrying an average traffic demand of 1 vehicle movement every 5 minutes.
2. Eaglecrest Drive just north of Inglis Road had measured two way traffic volumes of 9 vehicles per hour at Inglis Road and 42 vehicles per hour at O'Shea Road. Just south of O'Shea Road, Eaglecrest Drive was carrying an average traffic demand of 1 vehicle movement every 1.4 minutes

- Winn Road had measured two way traffic volumes of 14 vehicles per hour just east of Stewart Road and 139 vehicles per hour just west of Gower Point Road. Just west of Gower Point Road, Winn Road was carrying an average traffic demand of 2.3 vehicle movements every 1.4 minutes

CTS is forecasting in the traffic impact study that the proposed development will at full development generate 42 vehicle trips during the weekday morning peak hour (i.e. 10 inbound and 32 outbound) and 50 vehicle trips during the weekday afternoon peak hour (i.e. 32 inbound and 18 outbound). Therefore, the weekday afternoon peak hour will generate the highest volume of traffic for the development of which 36 vehicles per hour (i.e. 23 inbound and 13 outbound) will use the upper driveway on Eaglecrest Drive and 14 vehicles per hour (i.e. 9 inbound and 5 outbound) will use the lower driveway. This is illustrated on **FIGURE 2**.

FIGURE 2
SITE TRAFFIC VOLUMES FOR THE WEEKDAY AFTERNOON PEAK HOUR



As stated in the traffic impact study, the threshold of the Government of British Columbia for determining when a development triggers a formal traffic impact assessment is when a site generates 100 or more vehicles during the design hour. Therefore, the proposed development does not meet this requirement as the maximum volume forecast is only 50 vehicles, which is 50% of the warrant.

For the upper driveway, the forecast peak volume of 36 vehicles per hour is equivalent to an average of 1 vehicle movement every 1.7 minutes. For the lower driveway with a peak volume of 14 vehicles per hour forecast, this is equivalent to an average of 1 vehicle movement every 4.3 minutes. Therefore from a traffic engineering perspective, the site generated traffic volumes are considered negligible and any traffic impacts associated with this development would be localized to just the site access.

In reviewing **FIGURE 2**, the following commentary can be made on the site accesses:

1. For Inglis Drive between Eaglecrest Drive and Shaw Road, the site is forecast to add 18 vehicles per hour during the weekday afternoon peak hour (i.e. 7 westbound and 11 eastbound). This is equivalent to an average of 1 vehicle movement every 3.3 minutes combined for both directions.
2. For Eaglecrest Drive between Inglis Drive and O'Shea Road, the site is forecast to add 18 vehicles per hour as well during the weekday afternoon peak hour (i.e. 6 northbound and 12 southbound). This is equivalent to an average of 1 vehicle movement every 3.3 minutes combined for both directions.
3. For Winn Road between Stewart Road and Abbs Road, the site is forecast to add 14 vehicles per hour during the weekday afternoon peak hour (i.e. 9 westbound and 5 eastbound). This is equivalent to an average of 1 vehicle movement every 4.3 minutes combined for both directions.

Please call the undersigned should you have any questions or comments regarding the contents of this report.

Yours truly,

CREATIVE TRANSPORTATION SOLUTIONS LTD.



Jan O. Voss, P.Eng., PTOE
President

Attachment



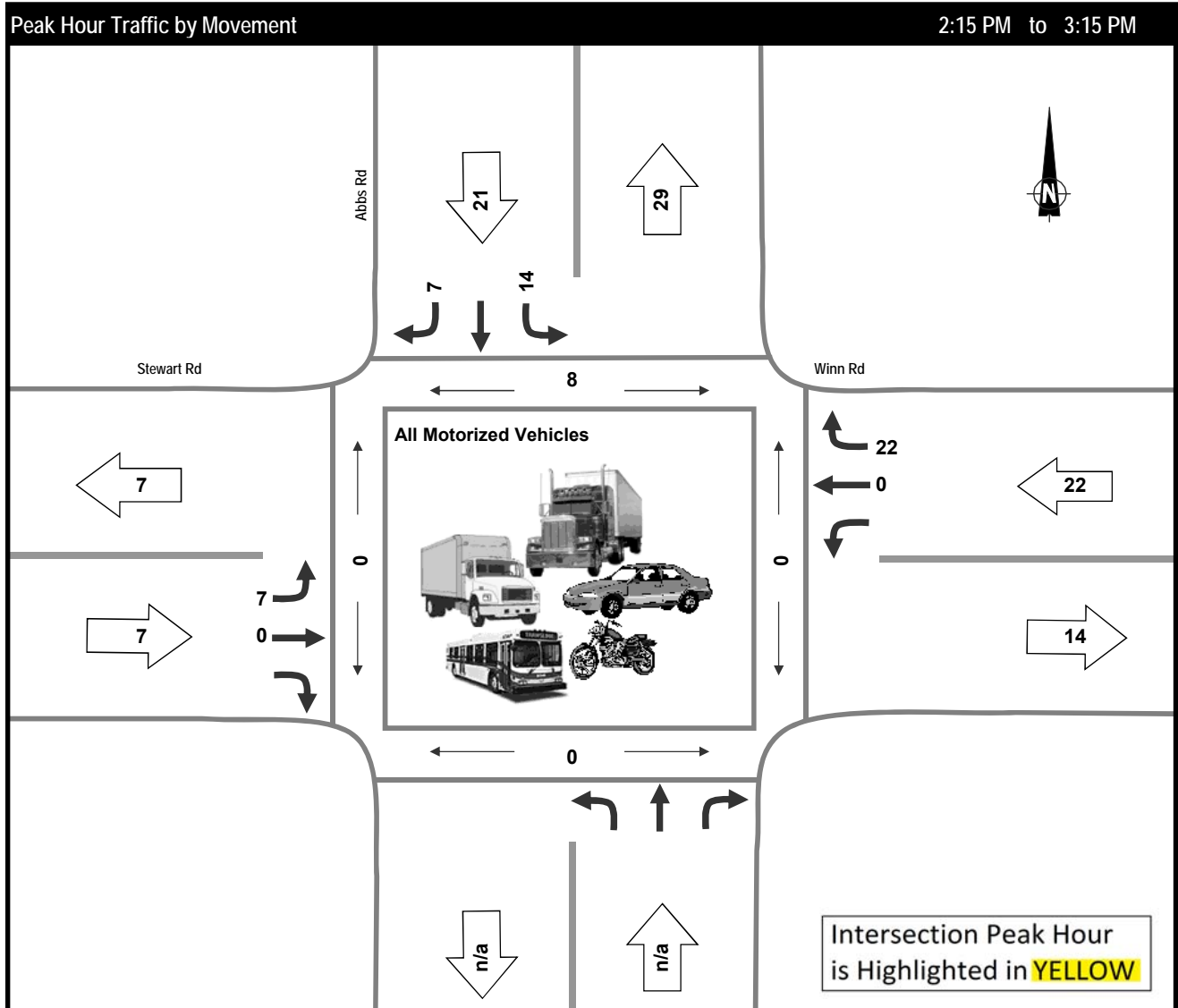
Vehicle Classification Summary

Project: #5602: 464 Eaglecrest Drive - Traffic Engineering Services
Municipality: Gibsons, BC
Weather: Clear, Sunny

Time Period	Entering Intersection	Vehicle Classification				Total
		Passenger Cars	Heavy Vehicles (3 or more axles)			
Morning	Volume					
	%					
Midday	Volume					
	%					
Afternoon (14:00 - 17:30)	Volume	138	0		138	
	%	100.0%	0.0%		100.0%	
Total (3.5 Hours)	Volume	138	0		138	
	%	100.0%	0.0%		100.0%	

Project: #5602: 464 Eaglecrest Drive - Traffic Engineering Services
 Municipality: Gibsons, BC
 Weather: Clear, Sunny
 Vehicle Class: All Motorized Vehicles

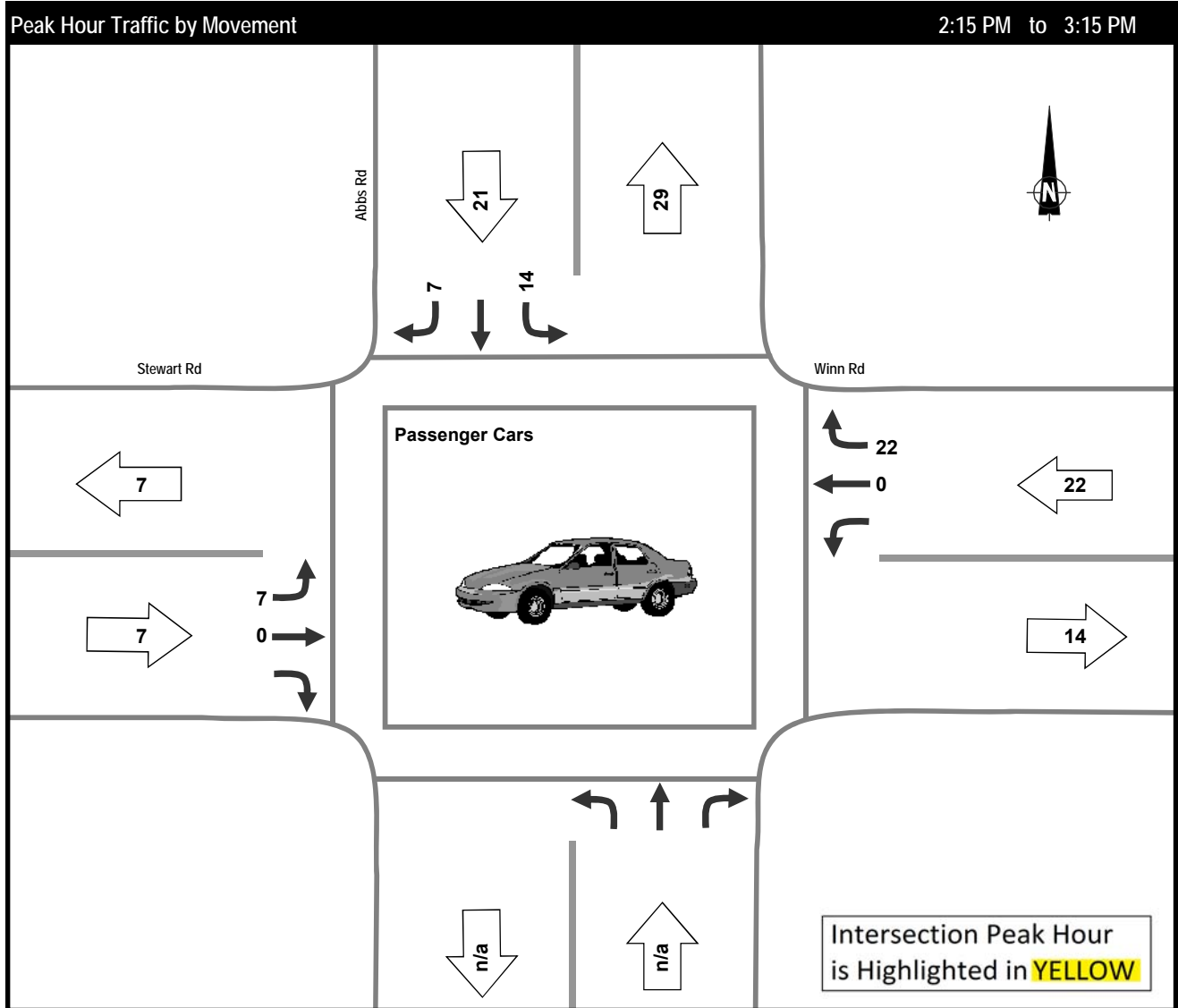
Afternoon Peak Period



Time	NORTH Approach			SOUTH Approach			WEST Approach			EAST Approach			PEDESTRIANS				Total Volumes
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	N	S	W	E	
Peak Hour	14		7				7	0			0	22	8	0	0	0	50
PHF	0.70		0.58				0.58	0.00			0.00	0.61	0.33	0.00	0.00	0.00	0.78
Peak 15 X 4	20		12				12	0			0	36	24	0	0	0	64
Average Hour	11		6				7	1			1	15	5	0	1	0	41
Survey Total	38		21				23	2			3	51	17	0	5	1	138
14:00	1		3				3	0			0	2	0	0	0	0	9
14:15	2		2				1	0			0	3	2	0	0	0	8
14:30	4		2				3	0			0	6	0	0	0	0	15
14:45	3		3				1	0			0	4	0	0	0	0	11
15:00	5		0				2	0			0	9	6	0	0	0	16
15:15	1		1				1	0			0	1	1	0	0	0	4
15:30	2		0				4	0			0	2	2	0	0	0	8
15:45	1		2				1	0			1	2	1	0	0	0	7
16:00	4		0				1	0			0	5	0	0	2	0	10
16:15	3		0				1	0			1	3	0	0	2	0	8
16:30	1		2				2	0			0	3	2	0	0	1	8
16:45	3		2				1	2			1	3	1	0	0	0	12
17:00	5		2				0	0			0	5	1	0	0	0	12
17:15	3		2				2	0			0	3	1	0	1	0	10

Project: #5602: 464 Eaglecrest Drive - Traffic Engineering Services
 Municipality: Gibsons, BC
 Weather: Clear, Sunny
 Vehicle Class: Passenger Cars

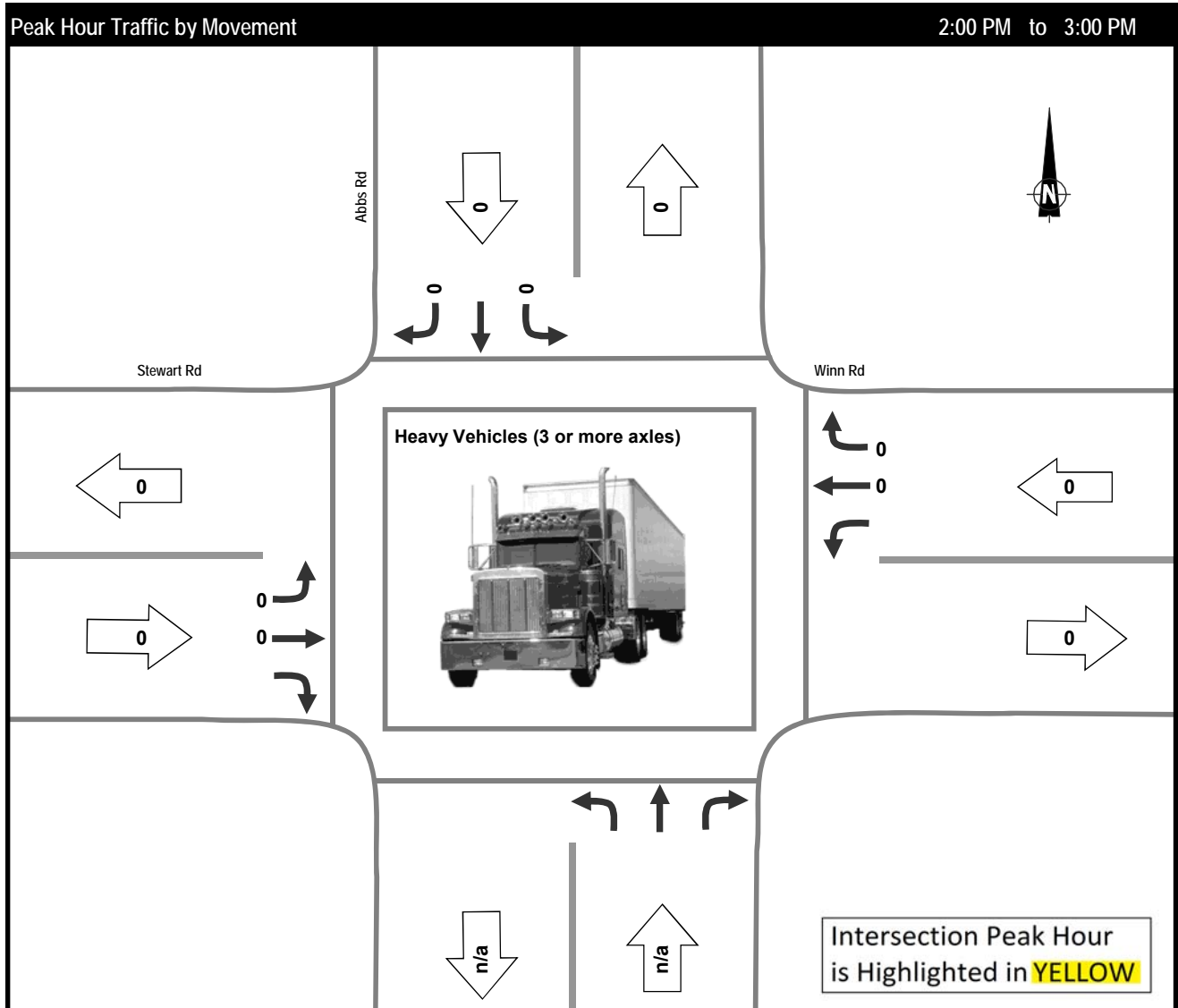
Afternoon Peak Period



Time	NORTH Approach			SOUTH Approach			WEST Approach			EAST Approach			PEDESTRIANS				Total Volumes
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	N	S	W	E	
Peak Hour	14		7				7	0			0	22					50
PHF	0.70		0.58				0.58	0.00			0.00	0.61					0.78
Peak 15 X 4	20		12				12	0			0	36					64
Average Hour	11		6				7	1			1	15					41
Survey Total	38		21				23	2			3	51					138
14:00	1		3				3	0			0	2					9
14:15	2		2				1	0			0	3					8
14:30	4		2				3	0			0	6					15
14:45	3		3				1	0			0	4					11
15:00	5		0				2	0			0	9					16
15:15	1		1				1	0			0	1					4
15:30	2		0				4	0			0	2					8
15:45	1		2				1	0			1	2					7
16:00	4		0				1	0			0	5					10
16:15	3		0				1	0			1	3					8
16:30	1		2				2	0			0	3					8
16:45	3		2				1	2			1	3					12
17:00	5		2				0	0			0	5					12
17:15	3		2				2	0			0	3					10

Project: #5602: 464 Eaglecrest Drive - Traffic Engineering Services
 Municipality: Gibsons, BC
 Weather: Clear, Sunny
 Vehicle Class: Heavy Vehicles (3 or more axles)

Afternoon Peak Period

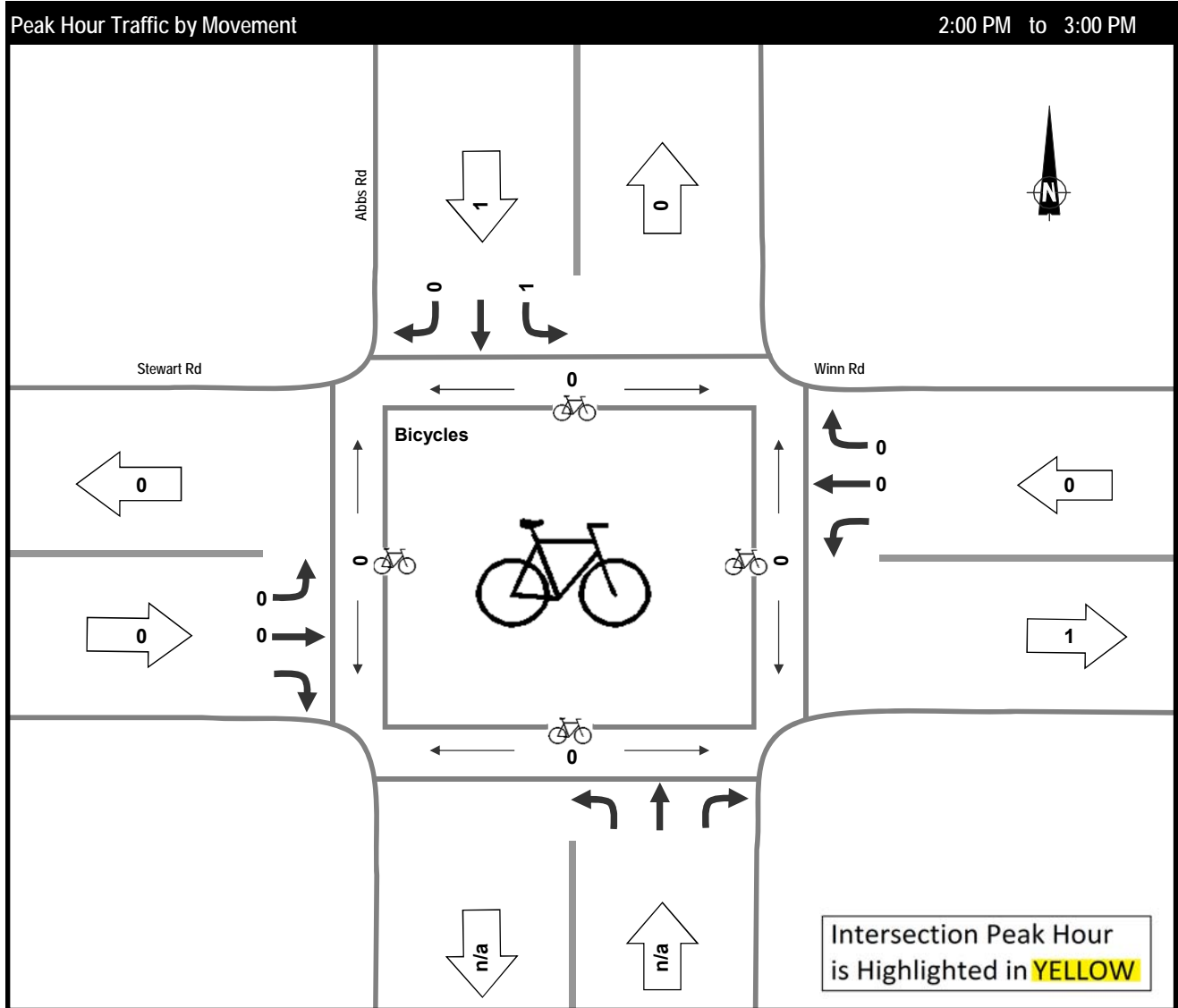


Time	NORTH Approach			SOUTH Approach			WEST Approach			EAST Approach			PEDESTRIANS				Total Volumes
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	N	S	W	E	
Peak Hour	0		0				0	0			0	0					0
PHF	0.00		0.00				0.00	0.00			0.00	0.00					0.00
Peak 15 X 4	0		0				0	0			0	0					0
Average Hour	0		0				0	0			0	0					0
Survey Total	0		0				0	0			0	0					0
14:00	0		0				0	0			0	0					0
14:15	0		0				0	0			0	0					0
14:30	0		0				0	0			0	0					0
14:45	0		0				0	0			0	0					0
15:00	0		0				0	0			0	0					0
15:15	0		0				0	0			0	0					0
15:30	0		0				0	0			0	0					0
15:45	0		0				0	0			0	0					0
16:00	0		0				0	0			0	0					0
16:15	0		0				0	0			0	0					0
16:30	0		0				0	0			0	0					0
16:45	0		0				0	0			0	0					0
17:00	0		0				0	0			0	0					0
17:15	0		0				0	0			0	0					0

Afternoon Peak Period

Project: #5602: 464 Eaglecrest Drive - Traffic Engineering Services
 Municipality: Gibsons, BC
 Weather: Clear, Sunny
 Vehicle Class: Bicycles

Note: Crosswalk bike volumes shown are cyclists who rode their bike across the crosswalk and are not included in the pedestrian volume totals



Time	NORTH Approach			SOUTH Approach			WEST Approach			EAST Approach			BIKES in X-WALKS				Total Volumes
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	N	S	W	E	
Peak Hour	1		0				0	0			0	0	0	0	0	0	1
PHF	0.25		0.00				0.00	0.00			0.00	0.00	0.00	0.00	0.00	0.00	0.25
Peak 15 X 4	4		0				0	0			0	0	0	0	0	0	4
Average Hour	0		0				0	0			0	0	0	0	0	0	0
Survey Total	1		0				0	0			1	0	0	0	0	0	2
14:00	0		0				0	0			0	0	0	0	0	0	0
14:15	0		0				0	0			0	0	0	0	0	0	0
14:30	1		0				0	0			0	0	0	0	0	0	1
14:45	0		0				0	0			0	0	0	0	0	0	0
15:00	0		0				0	0			0	0	0	0	0	0	0
15:15	0		0				0	0			0	0	0	0	0	0	0
15:30	0		0				0	0			0	0	0	0	0	0	0
15:45	0		0				0	0			0	0	0	0	0	0	0
16:00	0		0				0	0			0	0	0	0	0	0	0
16:15	0		0				0	0			0	0	0	0	0	0	0
16:30	0		0				0	0			0	0	0	0	0	0	0
16:45	0		0				0	0			0	0	0	0	0	0	0
17:00	0		0				0	0			1	0	0	0	0	0	1
17:15	0		0				0	0			0	0	0	0	0	0	0



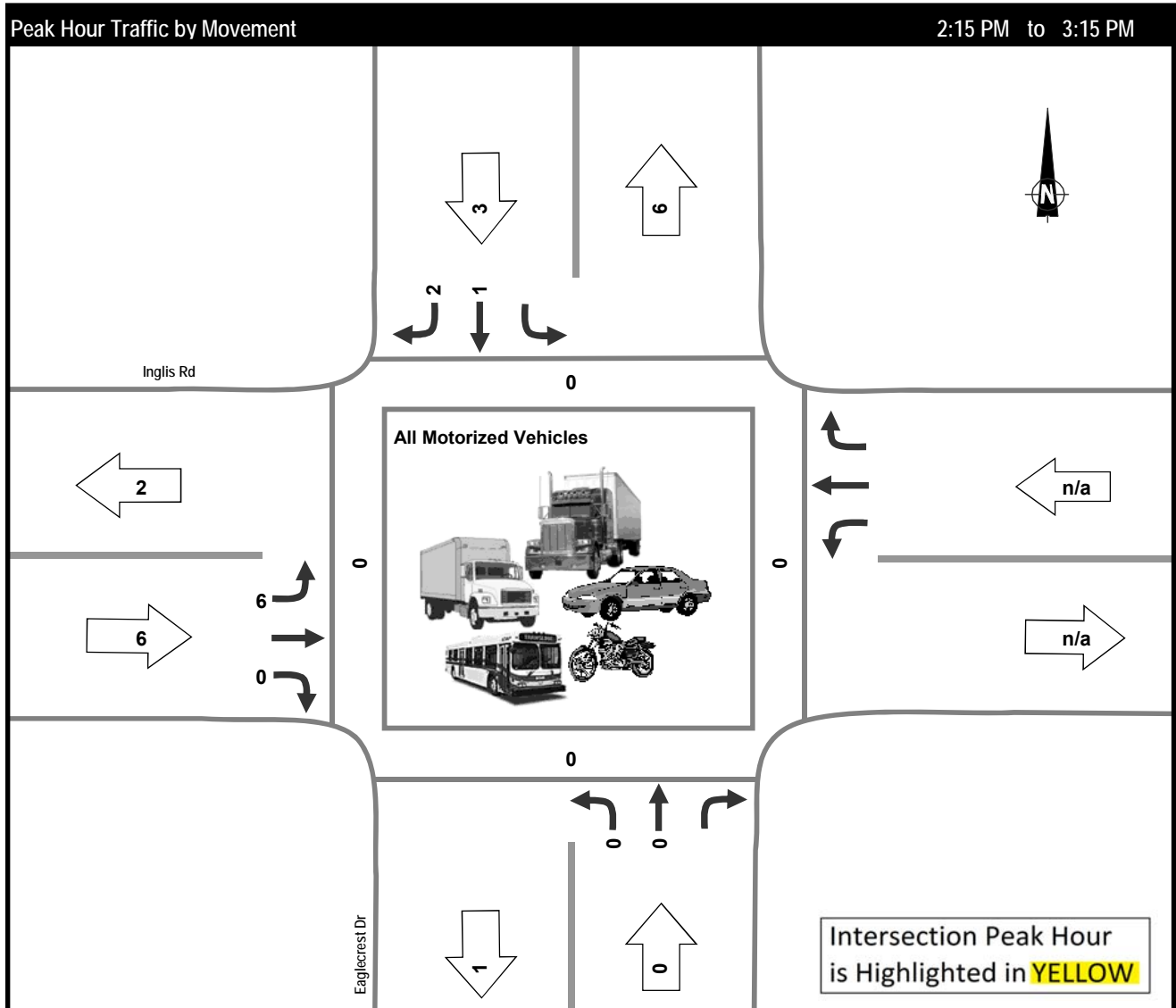
Vehicle Classification Summary

Project: #5602: 464 Eaglecrest Drive - Traffic Engineering Services
Municipality: Gibsons, BC
Weather: Sunny

Time Period	Entering Intersection	Vehicle Classification				Total
		Passenger Cars	Heavy Vehicles (3 or more axles)			
Morning	Volume					
	%					
Midday	Volume					
	%					
Afternoon (14:00 - 17:30)	Volume	26	0			26
	%	100.0%	0.0%			100.0%
Total (3.5 Hours)	Volume	26	0			26
	%	100.0%	0.0%			100.0%

Project: #5602: 464 Eaglecrest Drive - Traffic Engineering Services
Municipality: Gibsons, BC
Weather: Sunny
Vehicle Class: All Motorized Vehicles

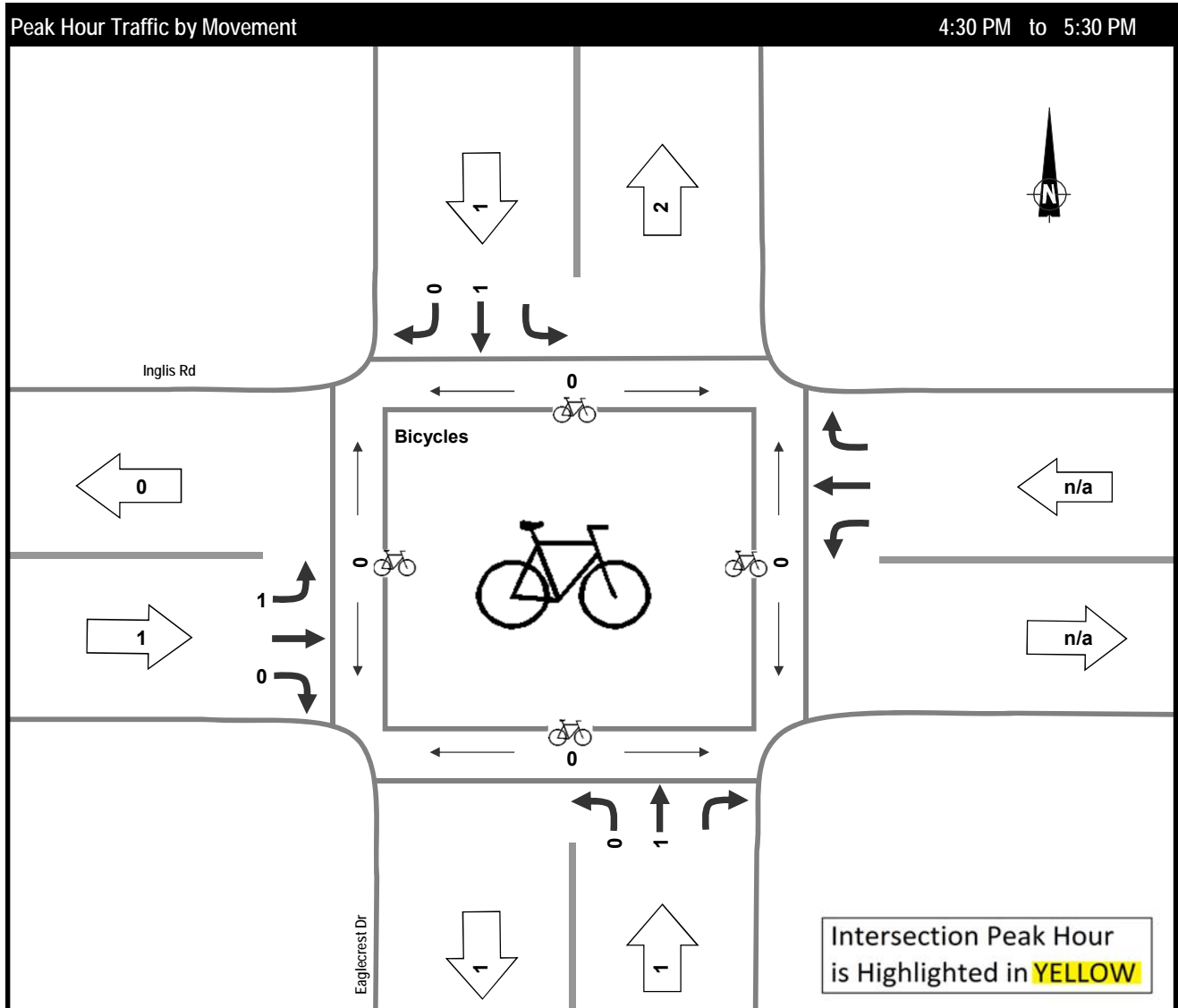
Afternoon Peak Period



Time	NORTH Approach			SOUTH Approach			WEST Approach			EAST Approach			PEDESTRIANS				Total Volumes
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	N	S	W	E	
Peak Hour		1	2	0	0		6		0				0	0	0	0	9
PHF		0.25	0.25	0.00	0.00		0.75		0.00				0.00	0.00	0.00	0.00	0.75
Peak 15 X 4		4	8	0	0		8		0				0	0	0	0	12
Average Hour		1	2	0	0		4		1				1	1	1	2	8
Survey Total		2	7	0	1		14		2				5	2	2	6	26
14:00		0	0	0	0		1		1				0	0	0	0	2
14:15		1	0	0	0		2		0				0	0	0	0	3
14:30		0	0	0	0		1		0				0	0	0	0	1
14:45		0	0	0	0		2		0				0	0	0	0	2
15:00		0	2	0	0		1		0				0	0	0	0	3
15:15		0	1	0	0		0		0				0	0	0	0	1
15:30		0	2	0	0		1		0				0	2	0	0	3
15:45		0	0	0	0		1		0				0	0	0	2	1
16:00		1	0	0	0		0		0				0	0	0	0	1
16:15		0	0	0	0		0		0				2	0	0	2	0
16:30		0	0	0	0		3		0				1	0	0	0	3
16:45		0	0	0	1		0		0				2	0	0	0	1
17:00		0	1	0	0		0		1				0	0	2	1	2
17:15		0	1	0	0		2		0				0	0	0	1	3

Project: #5602: 464 Eaglecrest Drive - Traffic Engineering Services
Municipality: Gibsons, BC
Weather: Sunny
Vehicle Class: Bicycles
Note: Crosswalk bike volumes shown are cyclists who rode their bike across the crosswalk and are not included in the pedestrian volume totals

Afternoon Peak Period



Time	NORTH Approach			SOUTH Approach			WEST Approach			EAST Approach			BIKES in X-WALKS				Total Volumes
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	N	S	W	E	
Peak Hour		1	0	0	1		1		0				0	0	0	0	3
PHF		0.25	0.00	0.00	0.25		0.25		0.00				0.00	0.00	0.00	0.00	0.25
Peak 15 X 4		4	0	0	4		4		0				0	0	0	0	12
Average Hour		0	0	0	1		1		0				0	0	0	0	2
Survey Total		1	0	0	2		2		0				0	0	0	0	5
14:00		0	0	0	0		0		0				0	0	0	0	0
14:15		0	0	0	0		0		0				0	0	0	0	0
14:30		0	0	0	0		0		0				0	0	0	0	0
14:45		0	0	0	0		0		0				0	0	0	0	0
15:00		0	0	0	0		0		0				0	0	0	0	0
15:15		0	0	0	1		0		0				0	0	0	0	1
15:30		0	0	0	0		0		0				0	0	0	0	0
15:45		0	0	0	0		1		0				0	0	0	0	1
16:00		0	0	0	0		0		0				0	0	0	0	0
16:15		0	0	0	0		0		0				0	0	0	0	0
16:30		0	0	0	0		0		0				0	0	0	0	0
16:45		0	0	0	0		0		0				0	0	0	0	0
17:00		0	0	0	0		0		0				0	0	0	0	0
17:15		1	0	0	1		1		0				0	0	0	0	3



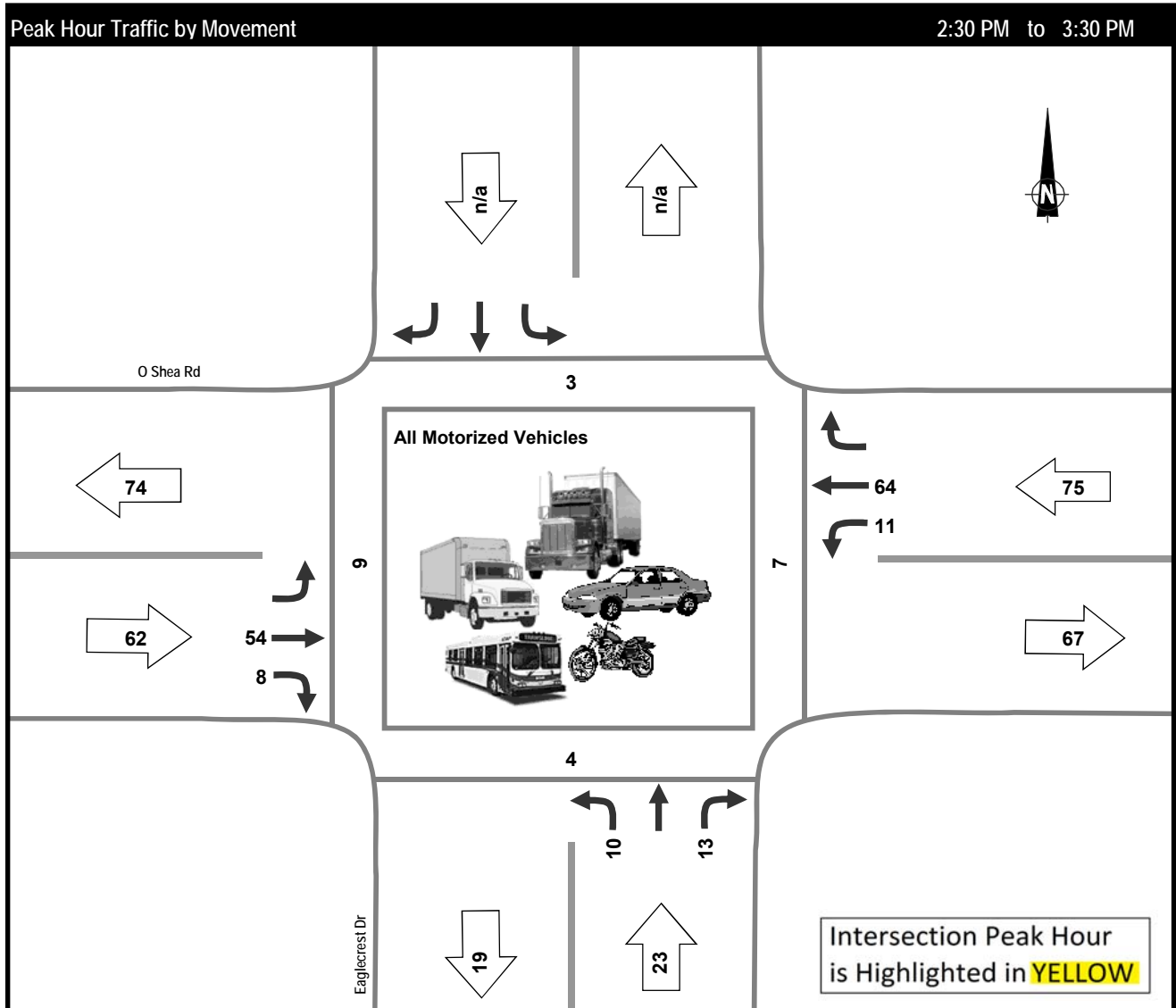
Vehicle Classification Summary

Project: #5602: 464 Eaglecrest Drive - Traffic Engineering Services
Municipality: Gibsons, BC
Weather: Sunny

Time Period	Entering Intersection	Vehicle Classification				Total
		Passenger Cars	Heavy Vehicles (3 or more axles)			
Morning	Volume					
	%					
Midday	Volume					
	%					
Afternoon (14:00 - 17:30)	Volume	420	0			420
	%	100.0%	0.0%			100.0%
Total (3.5 Hours)	Volume	420	0			420
	%	100.0%	0.0%			100.0%

Project: #5602: 464 Eaglecrest Drive - Traffic Engineering Services
 Municipality: Gibsons, BC
 Weather: Sunny
 Vehicle Class: All Motorized Vehicles

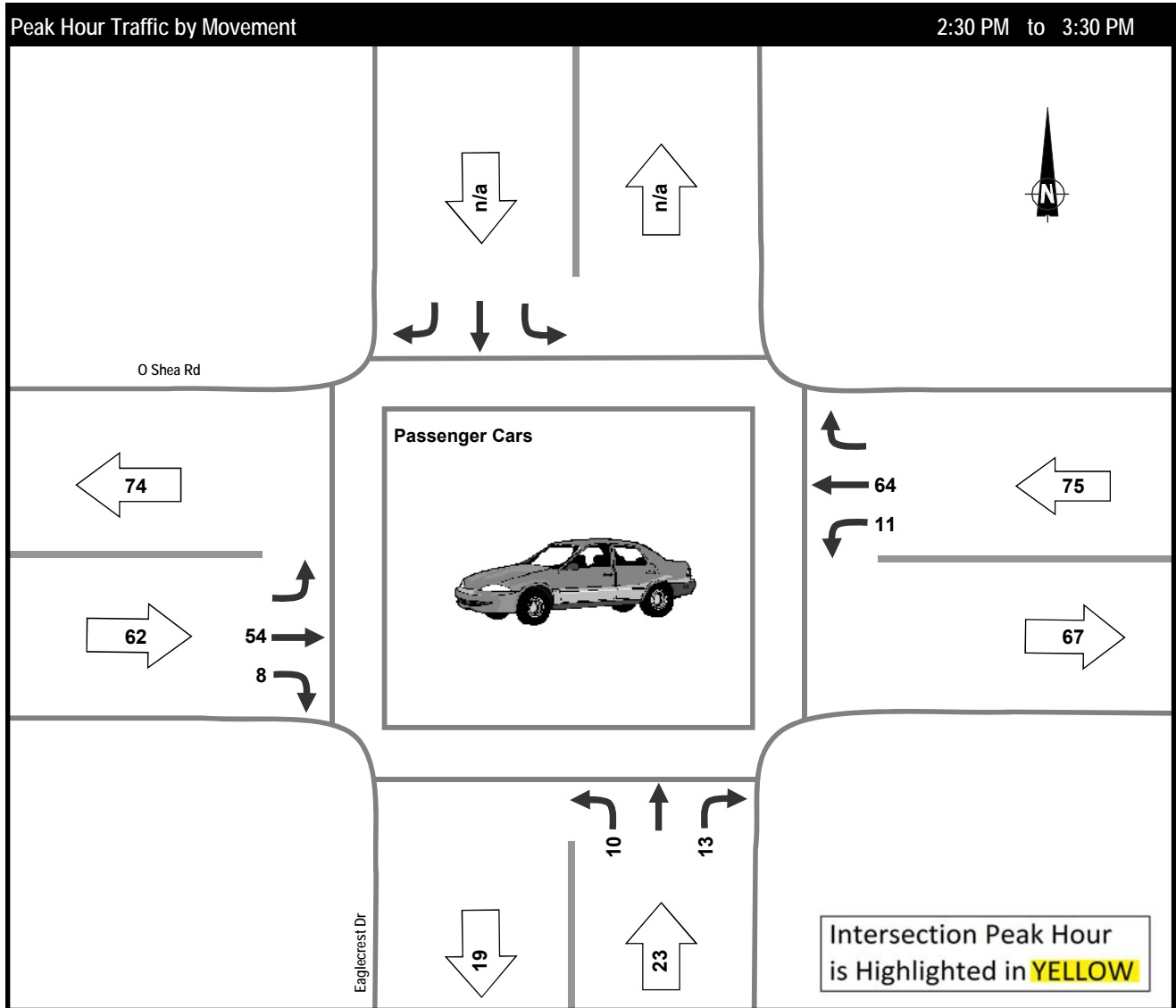
Afternoon Peak Period



Time	NORTH Approach			SOUTH Approach			WEST Approach			EAST Approach			PEDESTRIANS				Total Volumes
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	N	S	W	E	
Peak Hour				10		13	54	8	11	64			3	4	9	7	160
PHF				0.63		0.41	0.75	0.67	0.69	0.76			0.75	0.50	0.56	0.35	0.82
Peak 15 X 4				16		32	72	12	16	84			4	8	16	20	196
Average Hour				11		13	40	8	11	38			6	3	5	5	121
Survey Total				37		44	139	28	38	134			21	9	19	18	420
14:00				5		5	10	1	2	8			2	1	0	2	31
14:15				4		3	11	2	3	6			1	3	4	2	29
14:30				0		2	9	2	4	10			1	0	0	0	27
14:45				3		8	18	1	0	19			1	0	4	5	49
15:00				3		2	15	3	4	21			1	2	1	2	48
15:15				4		1	12	2	3	14			0	2	4	0	36
15:30				2		4	7	4	3	5			3	0	1	1	25
15:45				1		2	10	2	1	9			2	0	0	0	25
16:00				1		5	9	4	1	11			2	0	0	0	31
16:15				0		0	7	1	4	4			3	0	2	0	16
16:30				5		3	11	3	0	5			0	1	0	2	27
16:45				3		4	9	0	4	9			5	0	0	3	29
17:00				0		1	5	2	5	7			0	0	0	0	20
17:15				6		4	6	1	4	6			0	0	3	1	27

Project: #5602: 464 Eaglecrest Drive - Traffic Engineering Services
 Municipality: Gibsons, BC
 Weather: Sunny
 Vehicle Class: Passenger Cars

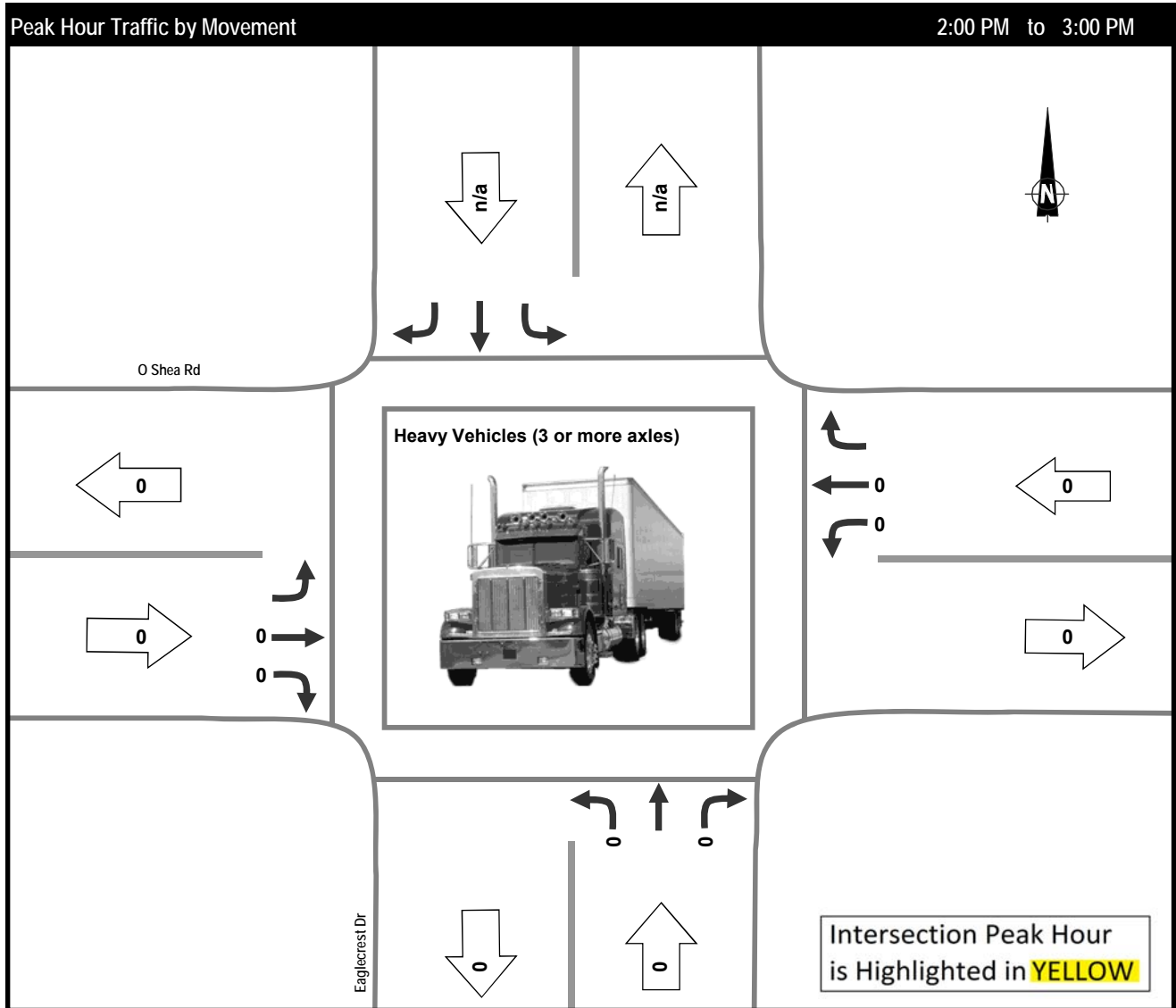
Afternoon Peak Period



Time	NORTH Approach			SOUTH Approach			WEST Approach			EAST Approach			PEDESTRIANS				Total Volumes	
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	N	S	W	E		
Peak Hour				10		13		54	8	11	64							160
PHF				0.63		0.41		0.75	0.67	0.69	0.76							0.82
Peak 15 X 4				16		32		72	12	16	84							196
Average Hour				11		13		40	8	11	38							121
Survey Total				37		44		139	28	38	134							420
14:00				5		5		10	1	2	8							31
14:15				4		3		11	2	3	6							29
14:30				0		2		9	2	4	10							27
14:45				3		8		18	1	0	19							49
15:00				3		2		15	3	4	21							48
15:15				4		1		12	2	3	14							36
15:30				2		4		7	4	3	5							25
15:45				1		2		10	2	1	9							25
16:00				1		5		9	4	1	11							31
16:15				0		0		7	1	4	4							16
16:30				5		3		11	3	0	5							27
16:45				3		4		9	0	4	9							29
17:00				0		1		5	2	5	7							20
17:15				6		4		6	1	4	6							27

Project: #5602: 464 Eaglecrest Drive - Traffic Engineering Services
 Municipality: Gibsons, BC
 Weather: Sunny
 Vehicle Class: Heavy Vehicles (3 or more axles)

Afternoon Peak Period

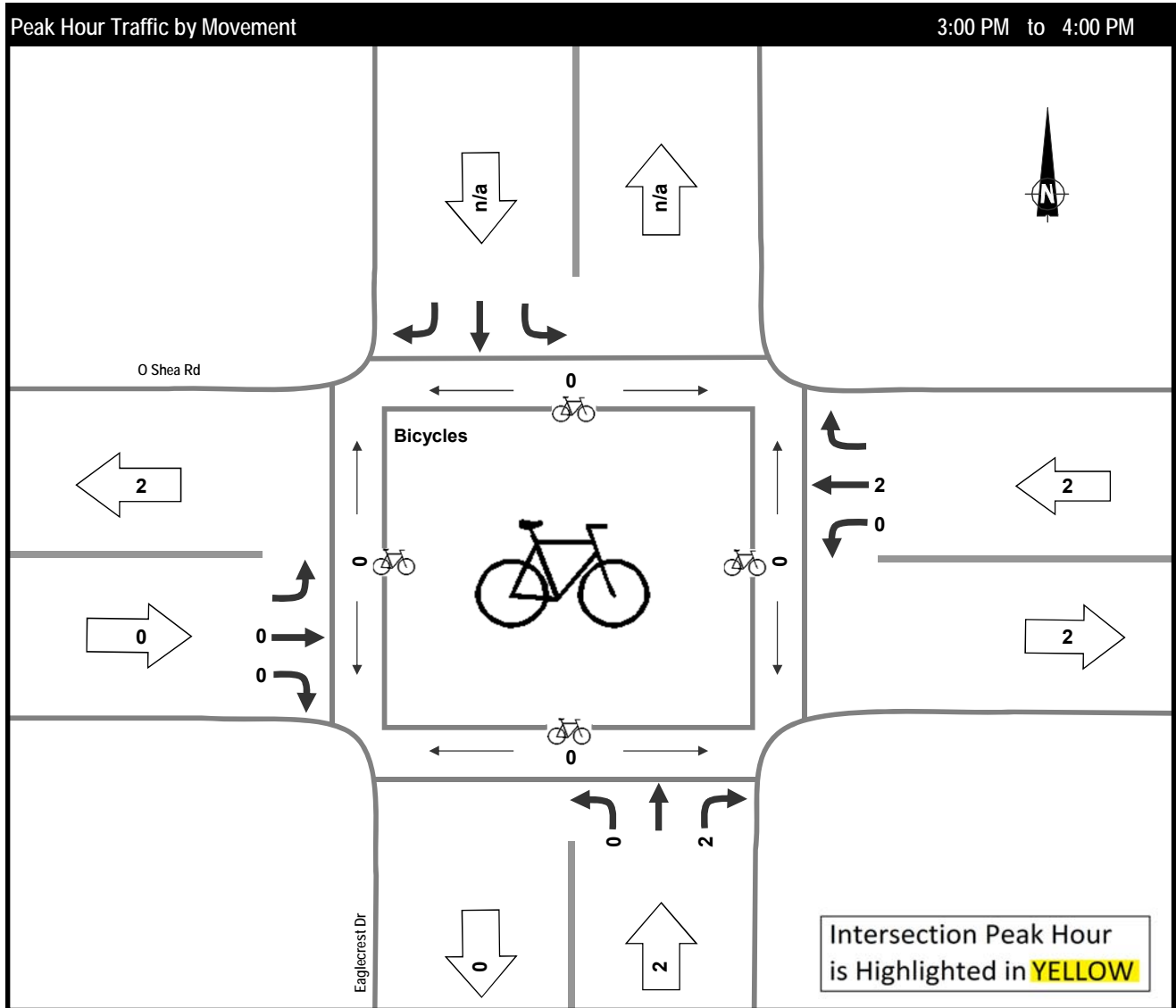


Time	NORTH Approach			SOUTH Approach			WEST Approach			EAST Approach			PEDESTRIANS				Total Volumes
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	N	S	W	E	
Peak Hour				0		0	0	0	0	0	0						0
PHF				0.00		0.00	0.00	0.00	0.00	0.00	0.00						0.00
Peak 15 X 4				0		0	0	0	0	0	0						0
Average Hour				0		0	0	0	0	0	0						0
Survey Total				0		0	0	0	0	0	0						0
14:00				0		0	0	0	0	0	0						0
14:15				0		0	0	0	0	0	0						0
14:30				0		0	0	0	0	0	0						0
14:45				0		0	0	0	0	0	0						0
15:00				0		0	0	0	0	0	0						0
15:15				0		0	0	0	0	0	0						0
15:30				0		0	0	0	0	0	0						0
15:45				0		0	0	0	0	0	0						0
16:00				0		0	0	0	0	0	0						0
16:15				0		0	0	0	0	0	0						0
16:30				0		0	0	0	0	0	0						0
16:45				0		0	0	0	0	0	0						0
17:00				0		0	0	0	0	0	0						0
17:15				0		0	0	0	0	0	0						0

Project: #5602: 464 Eaglecrest Drive - Traffic Engineering Services
 Municipality: Gibsons, BC
 Weather: Sunny
 Vehicle Class: Bicycles

Afternoon Peak Period

Note: Crosswalk bike volumes shown are cyclists who rode their bike across the crosswalk and are not included in the pedestrian volume totals



Time	NORTH Approach			SOUTH Approach			WEST Approach			EAST Approach			BIKES in X-WALKS				Total Volumes
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	N	S	W	E	
Peak Hour				0		2		0	0	0	2		0	0	0	0	4
PHF				0.00		0.50		0.00	0.00	0.00	0.50		0.00	0.00	0.00	0.00	0.50
Peak 15 X 4				0		4		0	0	0	4		0	0	0	0	8
Average Hour				0		1		0	0	0	1		0	0	0	0	2
Survey Total				0		2		0	0	0	2		0	0	0	0	4
14:00				0		0		0	0	0	0		0	0	0	0	0
14:15				0		0		0	0	0	0		0	0	0	0	0
14:30				0		0		0	0	0	0		0	0	0	0	0
14:45				0		0		0	0	0	0		0	0	0	0	0
15:00				0		0		0	0	0	0		0	0	0	0	0
15:15				0		1		0	0	0	1		0	0	0	0	2
15:30				0		0		0	0	0	0		0	0	0	0	0
15:45				0		1		0	0	0	1		0	0	0	0	2
16:00				0		0		0	0	0	0		0	0	0	0	0
16:15				0		0		0	0	0	0		0	0	0	0	0
16:30				0		0		0	0	0	0		0	0	0	0	0
16:45				0		0		0	0	0	0		0	0	0	0	0
17:00				0		0		0	0	0	0		0	0	0	0	0
17:15				0		0		0	0	0	0		0	0	0	0	0



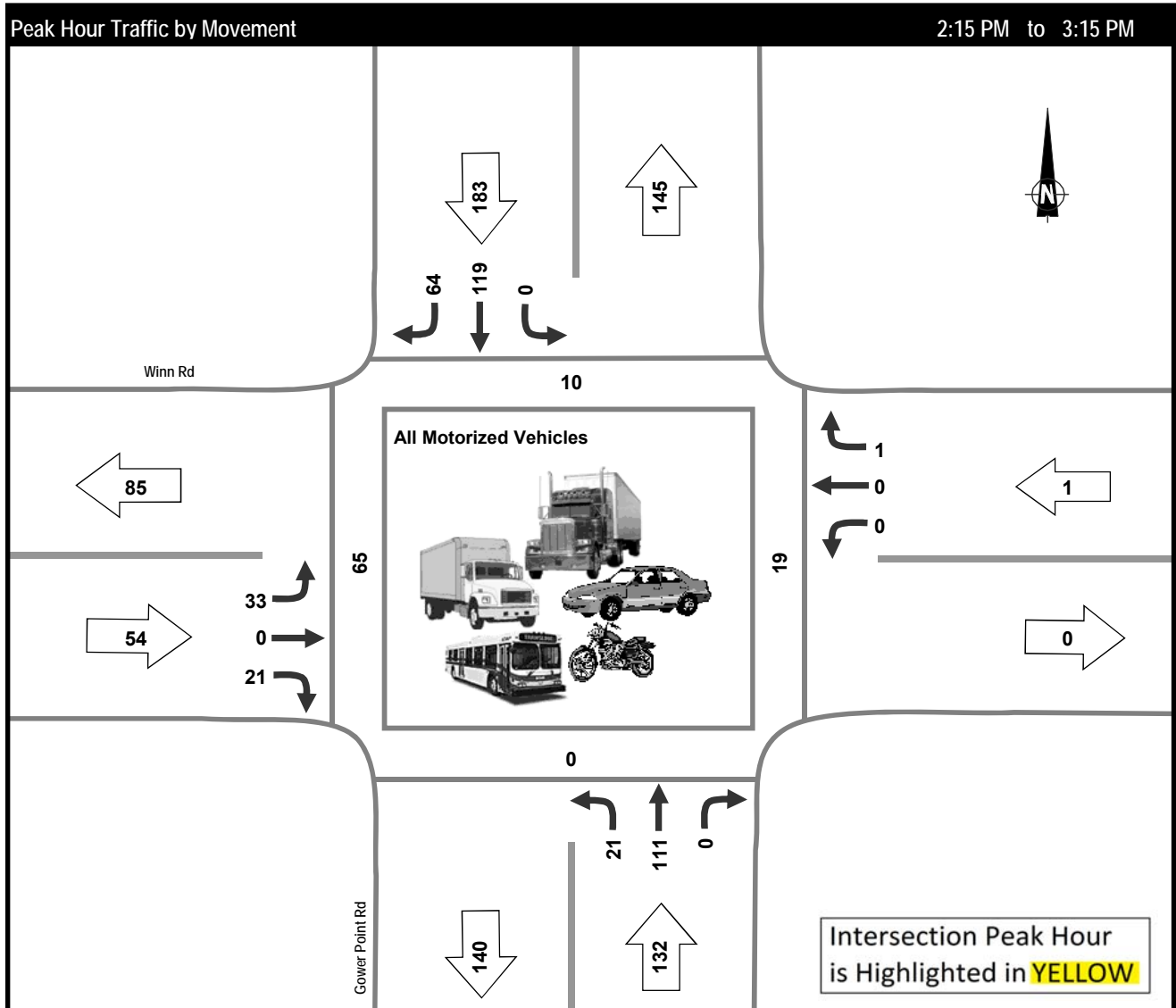
Vehicle Classification Summary

Project: #5602: 464 Eaglecrest Drive - Traffic Engineering Services
Municipality: Gibsons, BC
Weather: Sunny

Time Period	Entering Intersection	Vehicle Classification				Total
		Passenger Cars	Heavy Vehicles (3 or more axles)			
Morning	Volume					
	%					
Midday	Volume					
	%					
Afternoon (14:00 - 17:30)	Volume	1,152	12			1,164
	%	99.0%	1.0%			100.0%
Total (3.5 Hours)	Volume	1,152	12			1,164
	%	99.0%	1.0%			100.0%

Project: #5602: 464 Eaglecrest Drive - Traffic Engineering Services
Municipality: Gibsons, BC
Weather: Sunny
Vehicle Class: All Motorized Vehicles

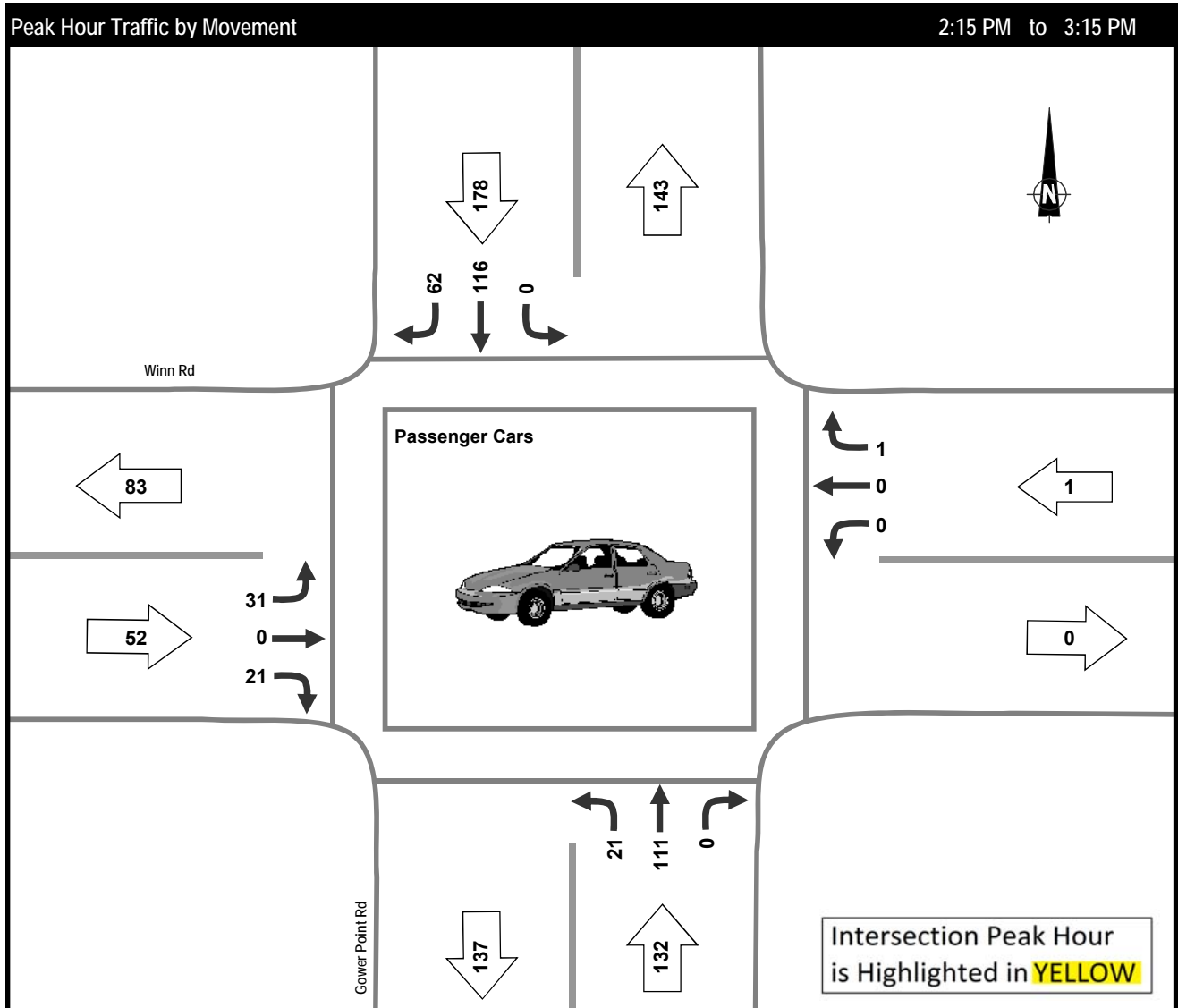
Afternoon Peak Period



Time	NORTH Approach			SOUTH Approach			WEST Approach			EAST Approach			PEDESTRIANS				Total Volumes
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	N	S	W	E	
Peak Hour	0	119	64	21	111	0	33	0	21	0	0	1	10	0	65	19	370
PHF	0.00	0.80	0.70	0.88	0.90	0.00	0.92	0.00	0.58	0.00	0.00	0.25	0.42	0.00	0.63	0.30	0.89
Peak 15 X 4	0	148	92	24	124	0	36	0	36	0	0	4	24	0	104	64	416
Average Hour	0	115	44	20	106	0	30	0	17	0	0	0	14	2	68	28	332
Survey Total	0	403	153	71	370	1	105	0	59	1	0	1	50	6	237	97	1,164
14:00	0	20	11	6	18	0	6	0	3	0	0	0	14	1	34	36	64
14:15	0	31	16	5	25	0	9	0	1	0	0	0	4	0	26	1	87
14:30	0	20	15	6	30	0	9	0	9	0	0	0	0	0	10	1	89
14:45	0	37	23	5	25	0	9	0	5	0	0	0	0	0	9	1	104
15:00	0	31	10	5	31	0	6	0	6	0	0	1	6	0	20	16	90
15:15	0	25	7	3	29	1	9	0	2	0	0	0	4	0	23	5	76
15:30	0	28	9	6	34	0	13	0	8	0	0	0	7	0	34	3	98
15:45	0	33	13	5	32	0	3	0	3	0	0	0	2	1	16	5	89
16:00	0	17	8	8	29	0	10	0	4	0	0	0	2	2	13	7	76
16:15	0	32	11	5	32	0	7	0	0	1	0	0	1	0	12	7	88
16:30	0	21	9	5	26	0	8	0	7	0	0	0	2	1	13	6	76
16:45	0	25	4	7	29	0	9	0	3	0	0	0	4	1	9	3	77
17:00	0	48	12	3	13	0	4	0	4	0	0	0	1	0	12	0	84
17:15	0	35	5	2	17	0	3	0	4	0	0	0	3	0	6	6	66

Project: #5602: 464 Eaglecrest Drive - Traffic Engineering Services
Municipality: Gibsons, BC
Weather: Sunny
Vehicle Class: Passenger Cars

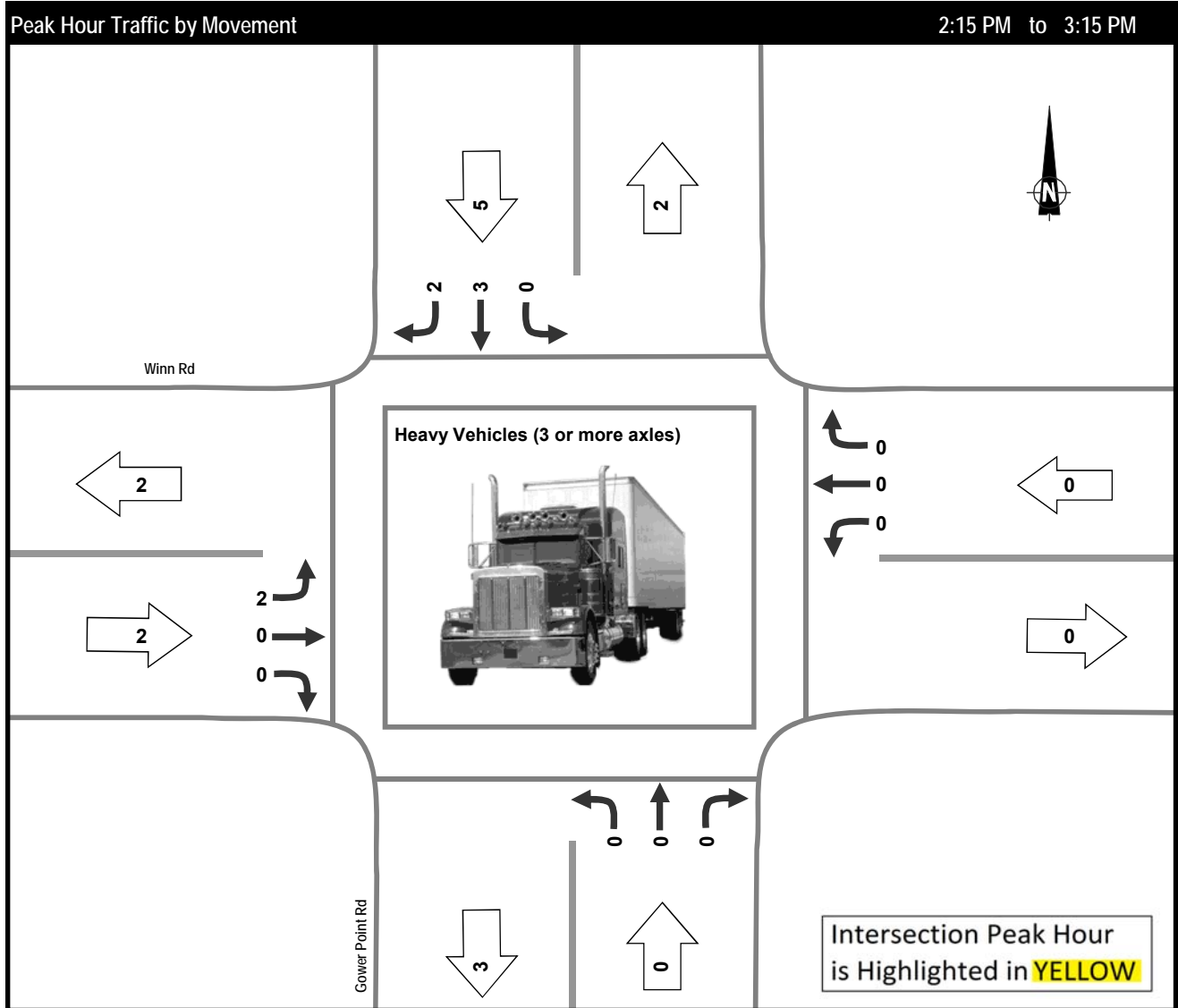
Afternoon Peak Period



Time	NORTH Approach			SOUTH Approach			WEST Approach			EAST Approach			PEDESTRIANS				Total Volumes
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	N	S	W	E	
Peak Hour	0	116	62	21	111	0	31	0	21	0	0	1					363
PHF	0.00	0.81	0.70	0.88	0.90	0.00	0.86	0.00	0.58	0.00	0.00	0.25					0.89
Peak 15 X 4	0	144	88	24	124	0	36	0	36	0	0	4					408
Average Hour	0	113	43	20	106	0	29	0	17	0	0	0					328
Survey Total	0	397	150	71	370	1	102	0	59	1	0	1					1,152
14:00	0	19	11	6	18	0	6	0	3	0	0	0					63
14:15	0	31	15	5	25	0	9	0	1	0	0	0					86
14:30	0	19	15	6	30	0	8	0	9	0	0	0					87
14:45	0	36	22	5	25	0	9	0	5	0	0	0					102
15:00	0	30	10	5	31	0	5	0	6	0	0	1					88
15:15	0	25	6	3	29	1	9	0	2	0	0	0					75
15:30	0	27	9	6	34	0	12	0	8	0	0	0					96
15:45	0	32	13	5	32	0	3	0	3	0	0	0					88
16:00	0	17	8	8	29	0	10	0	4	0	0	0					76
16:15	0	32	11	5	32	0	7	0	0	1	0	0					88
16:30	0	21	9	5	26	0	8	0	7	0	0	0					76
16:45	0	25	4	7	29	0	9	0	3	0	0	0					77
17:00	0	48	12	3	13	0	4	0	4	0	0	0					84
17:15	0	35	5	2	17	0	3	0	4	0	0	0					66

Project: #5602: 464 Eaglecrest Drive - Traffic Engineering Services
 Municipality: Gibsons, BC
 Weather: Sunny
 Vehicle Class: Heavy Vehicles (3 or more axles)

Afternoon Peak Period



Time	NORTH Approach			SOUTH Approach			WEST Approach			EAST Approach			PEDESTRIANS				Total Volumes
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	N	S	W	E	
Peak Hour	0	3	2	0	0	0	2	0	0	0	0	0					7
PHF	0.00	0.75	0.50	0.00	0.00	0.00	0.50	0.00	0.00	0.00	0.00	0.00					0.88
Peak 15 X 4	0	4	4	0	0	0	4	0	0	0	0	0					8
Average Hour	0	2	1	0	0	0	1	0	0	0	0	0					4
Survey Total	0	6	3	0	0	0	3	0	0	0	0	0					12
14:00	0	1	0	0	0	0	0	0	0	0	0	0					1
14:15	0	0	1	0	0	0	0	0	0	0	0	0					1
14:30	0	1	0	0	0	0	1	0	0	0	0	0					2
14:45	0	1	1	0	0	0	0	0	0	0	0	0					2
15:00	0	1	0	0	0	0	1	0	0	0	0	0					2
15:15	0	0	1	0	0	0	0	0	0	0	0	0					1
15:30	0	1	0	0	0	0	1	0	0	0	0	0					2
15:45	0	1	0	0	0	0	0	0	0	0	0	0					1
16:00	0	0	0	0	0	0	0	0	0	0	0	0					0
16:15	0	0	0	0	0	0	0	0	0	0	0	0					0
16:30	0	0	0	0	0	0	0	0	0	0	0	0					0
16:45	0	0	0	0	0	0	0	0	0	0	0	0					0
17:00	0	0	0	0	0	0	0	0	0	0	0	0					0
17:15	0	0	0	0	0	0	0	0	0	0	0	0					0



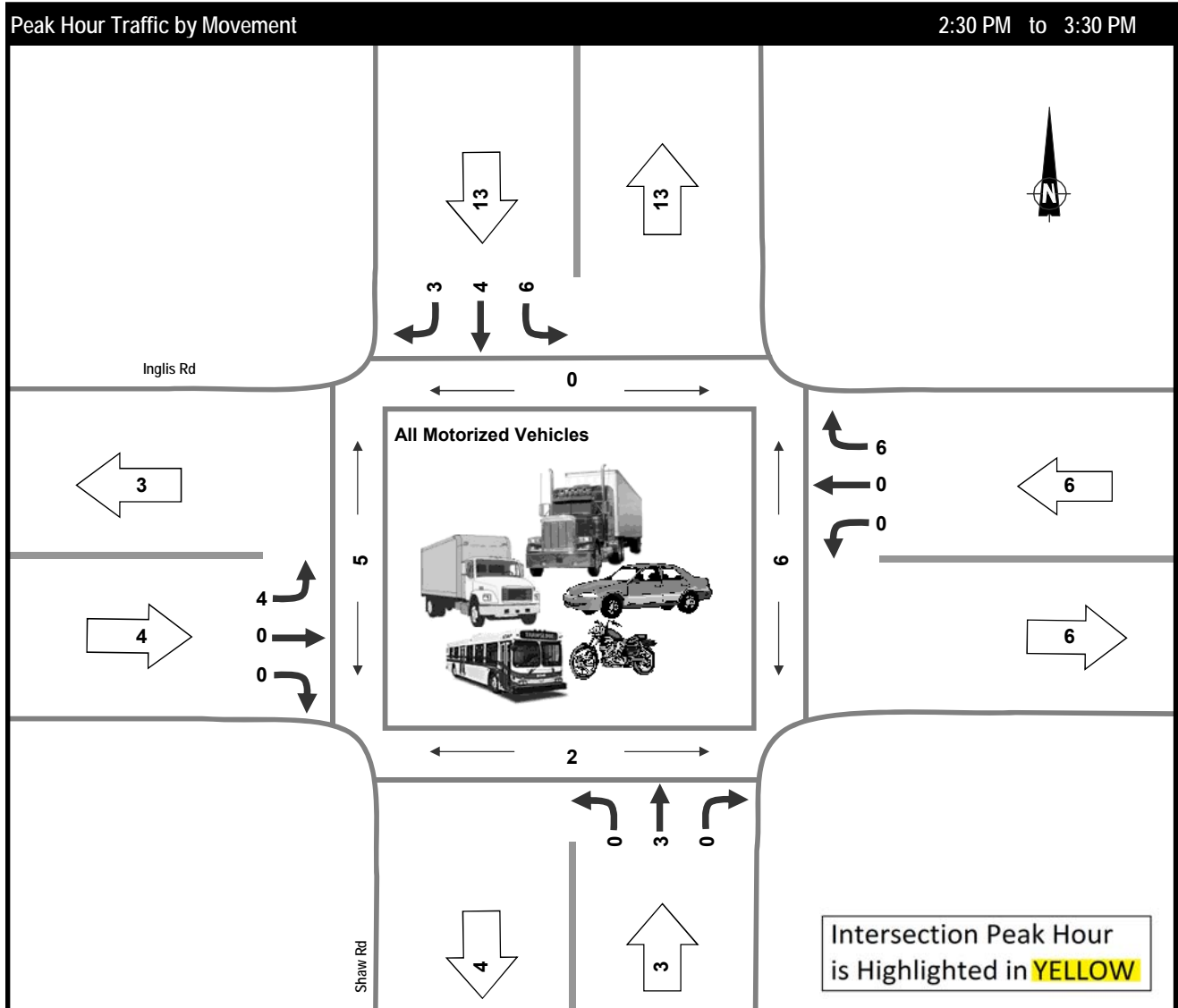
Vehicle Classification Summary

Project: #5602: 464 Eaglecrest Drive - Traffic Engineering Services
Municipality: Gibsons, BC
Weather: Clear, Sunny

Time Period	Entering Intersection	Vehicle Classification				Total
		Passenger Cars	Heavy Vehicles (3 or more axles)			
Morning	Volume					
	%					
Midday	Volume					
	%					
Afternoon (14:00 - 17:30)	Volume	68	0			68
	%	100.0%	0.0%			100.0%
Total (3.5 Hours)	Volume	68	0			68
	%	100.0%	0.0%			100.0%

Project: #5602: 464 Eaglecrest Drive - Traffic Engineering Services
Municipality: Gibsons, BC
Weather: Clear, Sunny
Vehicle Class: All Motorized Vehicles

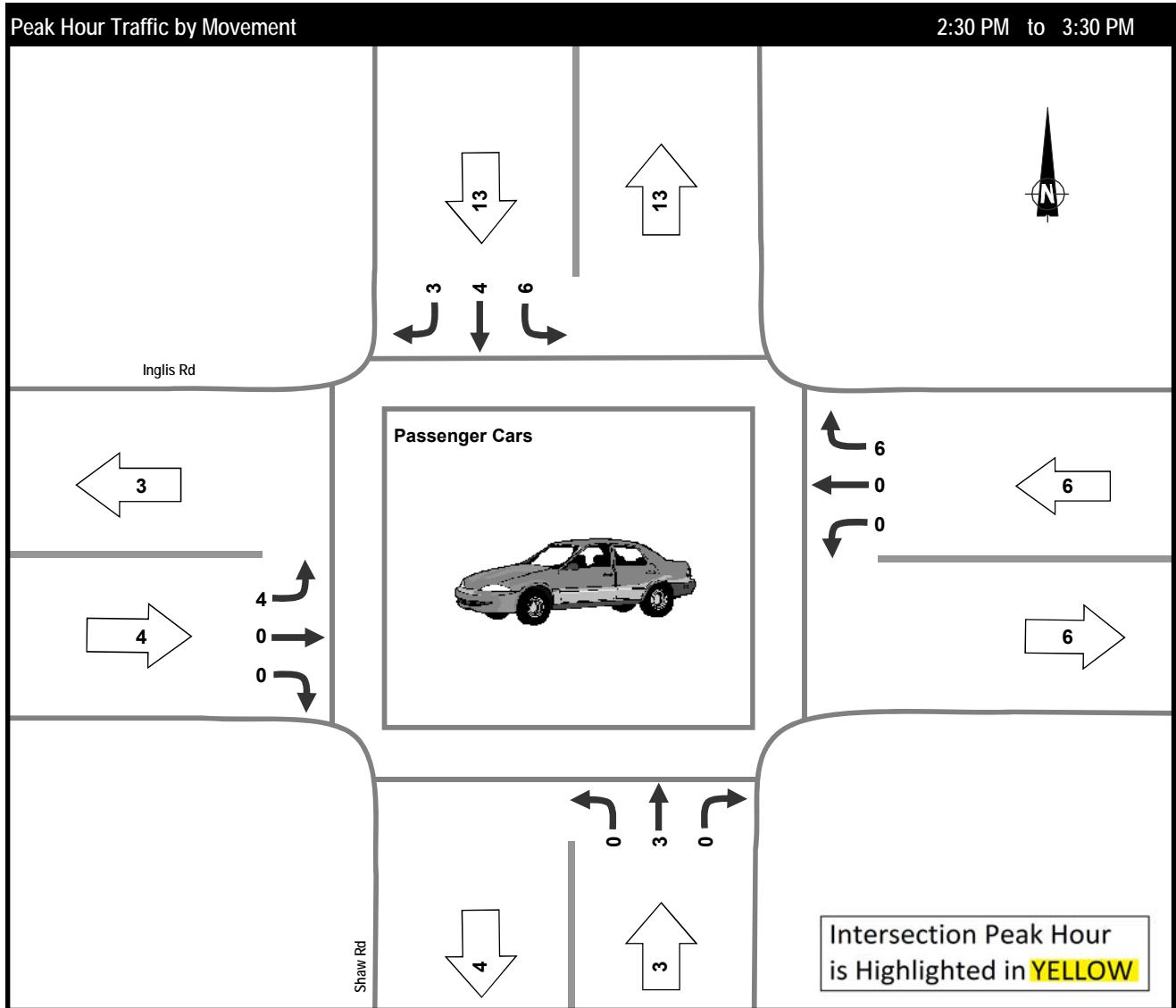
Afternoon Peak Period



Time	NORTH Approach			SOUTH Approach			WEST Approach			EAST Approach			PEDESTRIANS				Total Volumes
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	N	S	W	E	
Peak Hour	6	4	3	0	3	0	4	0	0	0	0	6	0	2	5	6	26
PHF	0.75	0.50	0.38	0.00	0.38	0.00	0.50	0.00	0.00	0.00	0.00	0.75	0.00	0.50	0.42	0.75	0.72
Peak 15 X 4	8	8	8	0	8	0	8	0	0	0	0	8	0	4	12	8	36
Average Hour	6	2	2	0	2	0	2	1	0	0	0	4	0	3	6	6	19
Survey Total	22	8	7	0	8	0	8	2	0	0	0	13	0	9	21	20	68
14:00	3	3	0	0	2	0	0	1	0	0	0	0	0	0	0	2	9
14:15	2	0	1	0	0	0	0	0	0	0	0	1	0	0	1	0	4
14:30	2	1	1	0	0	0	0	0	0	0	0	2	0	0	0	2	6
14:45	2	0	2	0	0	0	1	0	0	0	0	1	0	0	3	1	6
15:00	0	1	0	0	1	0	2	0	0	0	0	1	0	1	2	2	5
15:15	2	2	0	0	2	0	1	0	0	0	0	2	0	1	0	1	9
15:30	2	0	0	0	0	0	0	0	0	0	0	2	0	0	0	5	4
15:45	2	0	1	0	2	0	0	0	0	0	0	1	0	1	0	0	6
16:00	1	0	0	0	0	0	0	0	0	0	0	0	0	3	3	1	1
16:15	1	0	0	0	0	0	1	0	0	0	0	3	0	0	1	0	5
16:30	1	1	1	0	1	0	1	1	0	0	0	0	0	0	3	2	6
16:45	1	0	1	0	0	0	0	0	0	0	0	0	0	0	1	3	2
17:00	2	0	0	0	0	0	0	0	0	0	0	0	0	2	1	0	2
17:15	1	0	0	0	0	0	2	0	0	0	0	0	0	1	6	1	3

Project: #5602: 464 Eaglecrest Drive - Traffic Engineering Services
Municipality: Gibsons, BC
Weather: Clear, Sunny
Vehicle Class: Passenger Cars

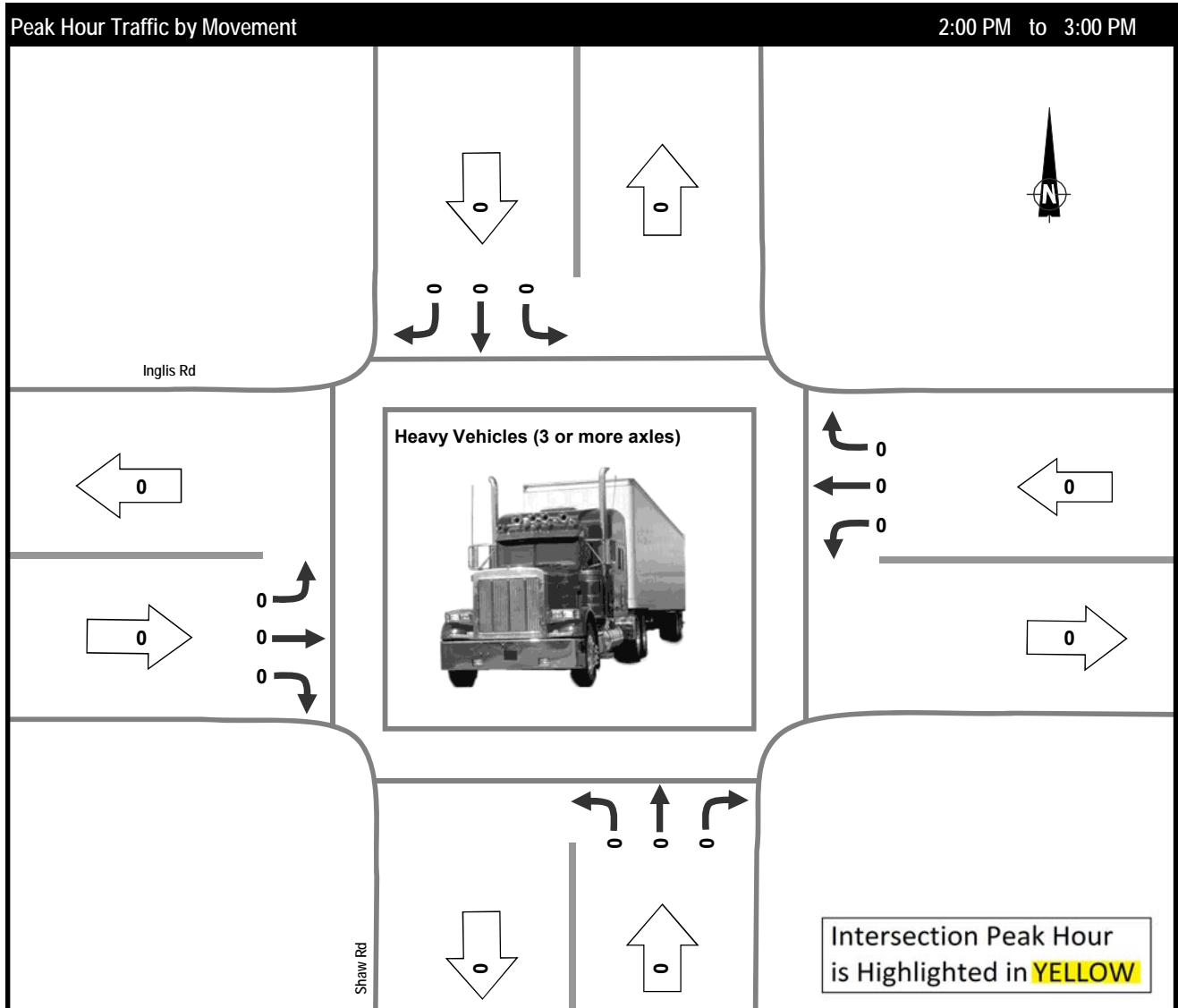
Afternoon Peak Period



Time	NORTH Approach			SOUTH Approach			WEST Approach			EAST Approach			PEDESTRIANS				Total Volumes
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	N	S	W	E	
Peak Hour	6	4	3	0	3	0	4	0	0	0	0	6					26
PHF	0.75	0.50	0.38	0.00	0.38	0.00	0.50	0.00	0.00	0.00	0.00	0.75					0.72
Peak 15 X 4	8	8	8	0	8	0	8	0	0	0	0	8					36
Average Hour	6	2	2	0	2	0	2	1	0	0	0	4					19
Survey Total	22	8	7	0	8	0	8	2	0	0	0	13					68
14:00	3	3	0	0	2	0	0	1	0	0	0	0					9
14:15	2	0	1	0	0	0	0	0	0	0	0	1					4
14:30	2	1	1	0	0	0	0	0	0	0	0	2					6
14:45	2	0	2	0	0	0	1	0	0	0	0	1					6
15:00	0	1	0	0	1	0	2	0	0	0	0	1					5
15:15	2	2	0	0	2	0	1	0	0	0	0	2					9
15:30	2	0	0	0	0	0	0	0	0	0	0	2					4
15:45	2	0	1	0	2	0	0	0	0	0	0	1					6
16:00	1	0	0	0	0	0	0	0	0	0	0	0					1
16:15	1	0	0	0	0	0	1	0	0	0	0	3					5
16:30	1	1	1	0	1	0	1	1	0	0	0	0					6
16:45	1	0	1	0	0	0	0	0	0	0	0	0					2
17:00	2	0	0	0	0	0	0	0	0	0	0	0					2
17:15	1	0	0	0	0	0	2	0	0	0	0	0					3

Project: #5602: 464 Eaglecrest Drive - Traffic Engineering Services
Municipality: Gibsons, BC
Weather: Clear, Sunny
Vehicle Class: Heavy Vehicles (3 or more axles)

Afternoon Peak Period

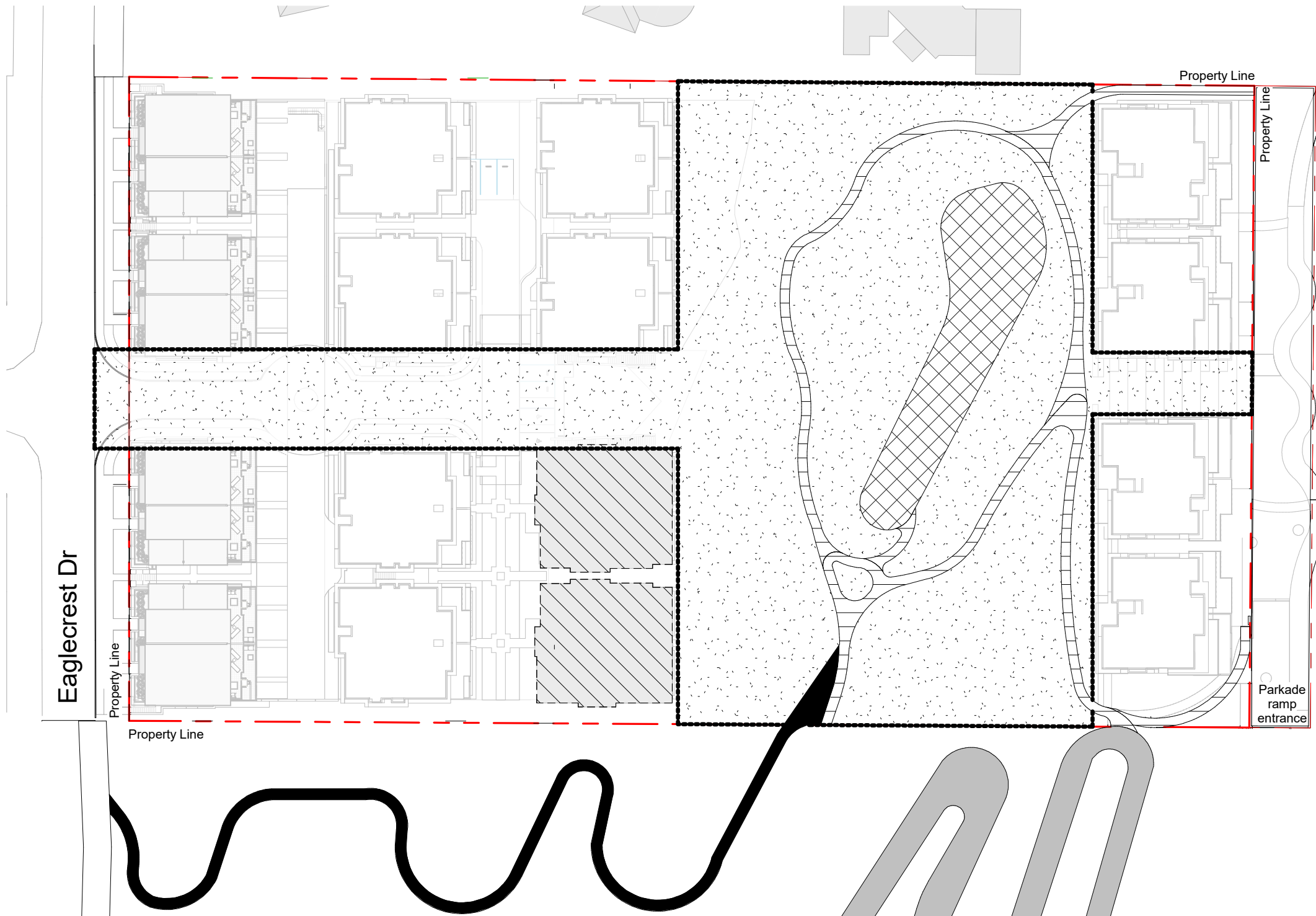








Time	NORTH Approach			SOUTH Approach			WEST Approach			EAST Approach			PEDESTRIANS				Total Volumes
	left	thru	right	left	thru	right	left	thru	right	left	thru	right	N	S	W	E	
Peak Hour	0	0	0	0	0	0	0	0	0	0	0	0					0
PHF	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00					0.00
Peak 15 X 4	0	0	0	0	0	0	0	0	0	0	0	0					0
Average Hour	0	0	0	0	0	0	0	0	0	0	0	0					0
Survey Total	0	0	0	0	0	0	0	0	0	0	0	0					0
14:00	0	0	0	0	0	0	0	0	0	0	0	0					0
14:15	0	0	0	0	0	0	0	0	0	0	0	0					0
14:30	0	0	0	0	0	0	0	0	0	0	0	0					0
14:45	0	0	0	0	0	0	0	0	0	0	0	0					0
15:00	0	0	0	0	0	0	0	0	0	0	0	0					0
15:15	0	0	0	0	0	0	0	0	0	0	0	0					0
15:30	0	0	0	0	0	0	0	0	0	0	0	0					0
15:45	0	0	0	0	0	0	0	0	0	0	0	0					0
16:00	0	0	0	0	0	0	0	0	0	0	0	0					0
16:15	0	0	0	0	0	0	0	0	0	0	0	0					0
16:30	0	0	0	0	0	0	0	0	0	0	0	0					0
16:45	0	0	0	0	0	0	0	0	0	0	0	0					0
17:00	0	0	0	0	0	0	0	0	0	0	0	0					0
17:15	0	0	0	0	0	0	0	0	0	0	0	0					0

Eaglecrest Drive

464 Eaglecrest Drive, Gibsons, BC

Schedule C



Key Legend	
	Buildings over 8.0m in height
	Area in blanket SRW
	Trail to be constructed on Town Land Location/alignment of trail/right of way subject to change
	Future Trail Location/alignment of trail/right of way subject to change
	Existing Trail
	Stormwater Detention Pond

② Schedule C
1/64" = 1'-0"